

Contents

1	Intr	oduction	1
2	Site	e Description	1
3	Ge	ological Model	2
	3.1	Regional Geology	2
	3.2	Geomorphology	2
	3.3	Geohazards	2
		3.3.1 Seismicity	2
		3.3.2 Liquefaction and Lateral Spreading	2
	3.4	Flooding	2
	3.5	ECan Boreholes	3
	3.6	Groundwater	5
	3.7	Site Investigation	5
	3.8	Site Seismic Class	5
4	Liq	uefaction Analysis	5
5	RM	A Section 106 Requirements and Suitability to Subdivide	6
6	Ge	otechnical Recommendations	6
	6.1	Earthworks	6
	6.2	Subdivision Roading	7
	6.3	Stormwater Control	7
	6.4	Foundations	7
7	Lim	itations	8
8	Ref	erences	9



Tables

Table 1: Generalised Summary of ECan Boreholes

Table 2: Summary of Subsurface Investigations

Figures

Figure 1: SDC Flooding map

Figure 2: Nearby ECan Borehole Locations

Appendices

Appendix 1: Development Plan

Appendix 2: ENGEO Southeast Rolleston Geotechnical Investigation

Appendix 3: Site Plan and Subsurface Investigation

Appendix 4: ECan Boreholes

Appendix 5: Statement of Professional Opinion



ENGEO Document Control:

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1 Introduction

ENGEO Ltd was requested by Broadfield Grange Limited to undertake a geotechnical investigation of the property at Broadfield Grange, Rolleston, Christchurch (herein referred to as 'the site'). This work has been carried out in accordance with our signed master agreement dated 3 June 2022 (ENGEO Ref: P2022.001.311).

The purpose of this assessment was to conceptualise a geological model of the site, assess the likely future land performance, comment on the suitability of the site for residential subdivision, address the requirements of Section 106 of the Resource Management Act (RMA), and provide recommendations for subdivision works and foundations for typical timber framed residential dwellings in support of subdivision consent.

Our scope of works included the following:

- Complete a desktop study of relevant available geotechnical and geological publications, including the NZ Geotechnical and Environment Canterbury Databases.
- Undertake a geotechnical site walkover.
- Organise service location for the proposed test locations.
- Undertake and log up to seven hand augers with Scala penetrometer tests to a maximum depth
 of approximately 0.9 m below ground level or practical refusal to assess the near surface
 material types and strength characteristics.
- Observe the excavation of up to seven test pits including geotechnical logging of the exposed soils. These pits were approximately 2.6 m deep, 3 m long and 1 m wide and loosely backfilled upon completion with the excavated soil. Recompaction was be accomplished by tamping with the excavator bucket.
- Preparation of this report outlining our findings on the ground conditions and the suitability of the site for residential subdivision including:
 - Geotechnical advice on the likely foundation Technical Category.
 - Conceptual foundation recommendations for typical timber framed residential dwellings.
 - o Address likely geohazards as required by Section 106 of the RMA.
 - o Provide a Statement of Professional Opinion on the Suitability of Land for Subdivision.

2 Site Description

The site comprises one property with a total area of approximately six hectares as depicted on the development plan presented in Appendix 1. ENGEO previously completed a plan change geotechnical report that included this site dated 9 December 2020, attached as Appendix 2. Our 9 December 2020 report should be read in conjunction with this report.



3 Geological Model

3.1 Regional Geology

The site has been regionally mapped by GNS (Forsyth et al., 2008) as being underlain by brownish grey river alluvium (Q2a).

3.2 Geomorphology

The site comprises relatively flat ground, with gentle undulations and depressions in some areas. As evident on aerial imagery (Canterbury Maps, 2019) and observed during our site walkover conducted on 2 June 2022, undulating and depressed ground can be attributed to paleo-channels, which traverse the site in a general northwest to southeast direction. Based on observations, sandy silt deposits with variable thickness are expected to have in-filled the paleo-channels where they have not remained as channel features.

3.3 Geohazards

3.3.1 Seismicity

There are no known or mapped faults in the immediate area of the site, however the site may be at risk of ground shaking induced by movement of proximal or distal faults.

The site is located between two recently discovered fault systems, the Greendale Fault and the Port Hills Fault, the ruptures of which initiated the ongoing Canterbury Earthquake Sequence (CES). The Greendale Fault has been mapped approximately 6 km northwest of the site and trends roughly east-west with a surface rupture length of approximately 28 km (GNS, 2015), while the Port Hills Fault remains unmapped as the fault did not rupture at the surface. Movement on the Port Hills Fault is believed to have extended to within 1 km to 2 km below ground surface.

Large regional areas of faulting (GNS, 2015) namely the Ashley Fault, Porters Pass - Amberley Fault Zone, and the Hope and Alpine Faults, are further afield but present a high seismic hazard to the Christchurch area due to the anticipated size of earthquakes generated. The largest of these faults is the Alpine Fault, which has a return period of 250 - 300 years and is expected to produce a M8 earthquake. The last rupture on the Alpine Fault is believed to have occurred in 1717 (Pettinga et al., 2001).

3.3.2 Liquefaction and Lateral Spreading

The site is located in an area mapped where "damaging liquefaction is unlikely" (NZGD Map CGD5140, 2012), and in a "zone of very low liquefaction potential" (GNS, 2006).

3.4 Flooding

The site is outside of any defined flood zones in the Selwyn District Council (SDC) Operative District Plan (SDC, 2015). The closest flood zone is the Lower Plains Flood Area which is approximately 4 km southeast of the site towards the Port Hills.

The Selwyn District Council have carried out computer-based flood modelling to predict the extent and depth of flooding that could happen during a one-in-200-year and a one-in-500-year flood. Based on this modelling, the water depth through the site may be up to 0.5 m deep but is generally less than 0.2 m (Figure 1) during the 500-year flood.



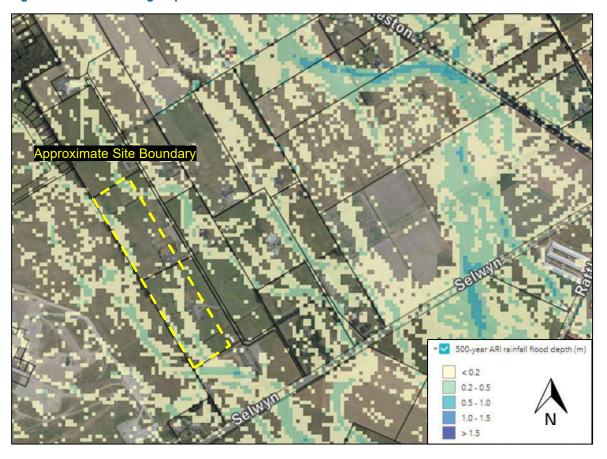


Figure 1: SDC Flooding map

From Canterbury Maps and Selwyn Council. Not to scale.

3.5 ECan Boreholes

We have reviewed deep ECan borehole logs located on the site (as shown in Figure 2) and have reviewed the monitoring well data from the monitoring wells on the site (part of the ECan Water Level Monitoring Network).

The logs for the wells located on the site, or close to the site boundary indicate the underlying soil generally comprises of gravels which extend to the bottom of the drill holes with isolated clay and silt layers at depth. The groundwater levels recorded in these wells are an average of approximately 10.6 m depth. The details for the wells are provided in Appendix 4 (including available drill logs).



Table 1: Generalised Summary of ECan Boreholes

ECan Borehole	Total Depth (m)	Initial Water Level Below Ground Level (m)	Generalised Borelog as Logged by Driller
M36_7975	37.5	10	Gravel to the maximum depth of testing with a 1 m thick silt layer from 25 m to 26 m depth.
M36_7976	36	10.2	Gravel to the maximum depth of drilling.
M36_8002	66	12.1	Gravel to the maximum depth of testing with 1 m thick clay layers from 26 m to 27 m depth and 32 m to 33 m.
M36_8009	36	11.8	Gravel to the maximum depth of drilling.
M36_8392	36	10	Gravel to the maximum depth of drilling.
Average Grou	ndwater depth	10.8	

Figure 2: Nearby ECan Borehole Locations



Aerial photograph sourced from Canterbury Maps. Not to scale.



3.6 Groundwater

Groundwater is recorded in the surrounding ECan boreholes at approximately 10 to 11 m depth.

3.7 Site Investigation

As part of the plan change report dated December 2020, thee test pits were completed on, or near the site and an additional seven test pits and seven hand auger investigations were undertaken as part of this assessment by ENGEO on 2 June 2021. These were completed to a maximum depth of 2.6 m below ground level.

The investigations revealed subsurface conditions across the site are consistent with the published geological mapping, as summarised in Table 2. Hand auger and test pit logs are attached as Appendix 3 of this report.

Table 2: Summary of Subsurface Investigations

Soil Type	Depth to Top of Layer (m)	General Layer Thickness (m)	Density / Consistency	Additional Comments
TOPSOIL	0.0	0.25	-	-
Sandy GRAVEL	0.3	Unknown	Medium Dense to Very Dense	Tightly packed and consistent across the site. Thin (<0.4 m) silty SAND or SAND layers encountered in the initial 0.5 m (up to 0.9 m) in some test pit locations.

3.8 Site Seismic Class

In accordance with NZS 1170.5:2004, Class D applies to this particular site, defining it as a 'deep soft soil site'.

4 Liquefaction Analysis

Owing to the nature of the subsurface materials and depth to groundwater at the site, we consider the potential for liquefaction and lateral spreading on the site to be very low.

We therefore consider future land performance to be in line with Technical Category 1 (TC1), whereby future land damage from liquefaction is unlikely, and ground settlements are expected to be within normally accepted tolerances.



5 RMA Section 106 Requirements and Suitability to Subdivide

Section 106 of the Resource Management Act 1991 states a consent authority may refuse to grant a subdivision consent, or may grant a consent subject to specific consent conditions if it considers that:

- There is a significant risk from natural hazards; or
- Sufficient provision has not been made for legal or physical access to each allotment to be created by the subdivision.

An assessment of the risk from natural hazards as required by the RMA includes the following:

- The likelihood of natural hazards occurring (whether individually or in combination);
- The material damage to land in respect of which the consent is sought, other land, or structures that would result from natural hazards; and
- Any likely subsequent use of the land in respect of which the consent is sought that would accelerate, worsen, or result in material damage of the kind referred to in paragraph (b).

We have assessed the risk of natural hazards at the site in accordance with Section 106 of the Resource Management Act (RMA) and considered the risk to the site from rockfall, inundation (debris), slope stability, subsidence, flooding and tsunami. Based on our observations and the nature of the site, its performance during the Canterbury Earthquake Sequence (CES), and the site's distance from the nearest significant watercourse, we consider it is unlikely for the site to be subject to natural hazards such as rockfall, inundation (debris), slope stability, subsidence and tsunami. As discussed in Section 3.4, we recommend that inundation hazard is considered during subdivision design. As such, the site is considered suitable for subdivision from a geotechnical perspective.

6 Geotechnical Recommendations

6.1 Earthworks

Earthworks carried out for the subdivision shall be in accordance with NZS 4404:2010, Land Development and Subdivision Infrastructure and NZS 4431:1989, Code of Practice for Earth filling for Residential Development. In particular, any areas to receive fill should be stripped of all vegetation, topsoil, non-engineered fill, soft or organic soils prior to fill placement.

Fill may comprise clean native sandy gravel or silty soils, or clean imported soils and / or granular fill, compacted to achieve no less than 95% of maximum dry density. Fill faces steeper than 2H:1V and higher than 600 mm should be retained and referred back to ENGEO. Although unlikely, where any springs or groundwater seeps are encountered, they should be intercepted with suitable drainage and discharged to a Council approved outlet.

All unretained batters of pond and stormwater drains constructed with the native sandy gravel material should be at an inclination no steeper than 1V:3H, with protection schemes in place to control erosion of the formed batters within the waterways.

A comprehensive earthworks specification should be provided to the earthworks contractor prior to starting excavations and an inspection / testing regime agreed, along with a robust erosion and sediment control plan.



6.2 Subdivision Roading

Vegetation, any organic or deleterious material, topsoil and non-engineered fill should be removed from under pavement areas prior to aggregate placement. Based on our observations during testing, we consider the native ground below the topsoil at the site should provide an adequate subgrade for the proposed pavement areas.

6.3 Stormwater Control

Concentrated stormwater flows from all impermeable areas must be collected and carried in sealed pipes to the Council system or an alternative disposal point subject to approval from Council. Uncontrolled stormwater must not be allowed to saturate the ground as this will potentially affect future foundation performance both statically and during future seismic activity.

6.4 Foundations

Foundations for future proposed residential dwellings within the subdivision may comprise shallow pad, strip, or slab foundations designed in accordance with the provisions of NZS 3604 Timber Framed Buildings.

Site specific testing will be required for Building Consent, to confirm the bearing materials and capacity. For preliminary design, we anticipate that a geotechnical Ultimate Bearing Capacity of 300 kPa may be assumed for foundations bearing on sandy gravel or engineered fill, below any topsoil. All topsoil shall be stripped from within building footprints; we anticipate this to be typically below 0.3 m depth based on our subsurface investigations.



7 Limitations

- i. We have prepared this report in accordance with the brief as provided. This report has been prepared for the use of our client, Broadfield Grange Limited, their professional advisers and the relevant Territorial Authorities in relation to the specified project brief described in this report. No liability is accepted for the use of any part of the report for any other purpose or by any other person or entity.
- ii. The recommendations in this report are based on the ground conditions indicated from published sources, site assessments and subsurface investigations described in this report based on accepted normal methods of site investigations. Only a limited amount of information has been collected to meet the specific financial and technical requirements of the client's brief and this report does not purport to completely describe all the site characteristics and properties. The nature and continuity of the ground between test locations has been inferred using experience and judgement and it should be appreciated that actual conditions could vary from the assumed model.
- iii. Subsurface conditions relevant to construction works should be assessed by contractors who can make their own interpretation of the factual data provided. They should perform any additional tests as necessary for their own purposes.
- iv. This Limitation should be read in conjunction with the Engineering NZ / ACENZ Standard Terms of Engagement.
- v. This report is not to be reproduced either wholly or in part without our prior written permission.

We trust that this information meets your current requirements. Please do not hesitate to contact the undersigned on (03) 328 9012 if you require any further information.

Report prepared by

Jed Watts

Engineering Geologist

Report reviewed by

Don Bruggers, CMEngNZ (CPEng)

Principal Engineer



8 References

Canterbury Maps, Groundwater. Retrieved June 2022 from http://canterburymaps.govt.nz/Viewer.

Forsyth, P., Barrell, D. J., & Jongens, R. (2008). Sheet 16 - Geology of the Christchurch Area 1:250,000. Lower Hutt: Institute of Geological and Nuclear Sciences.

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Selwyn District Council (2015), Selwyn District Council Operative District Plan. Retrieved December 2020, from http://www.selwyn.govt.nz/services/planning/district-plan.

Selwyn District Council (2015), Property Search. Retrieved June 2022, from https://www.selwyn.govt.nz/my-property/rates/search-properties.

The Ministry of Business, Innovation, and Employment. (2012). Guidance-Repairing and rebuilding houses affected by the Canterbury earthquakes. Christchurch: The Ministry of Business, Innovation, and Employment.

We also acknowledge the New Zealand GeoNet project and its sponsors EQC, GNS Science and LINZ, for providing data used in this report.

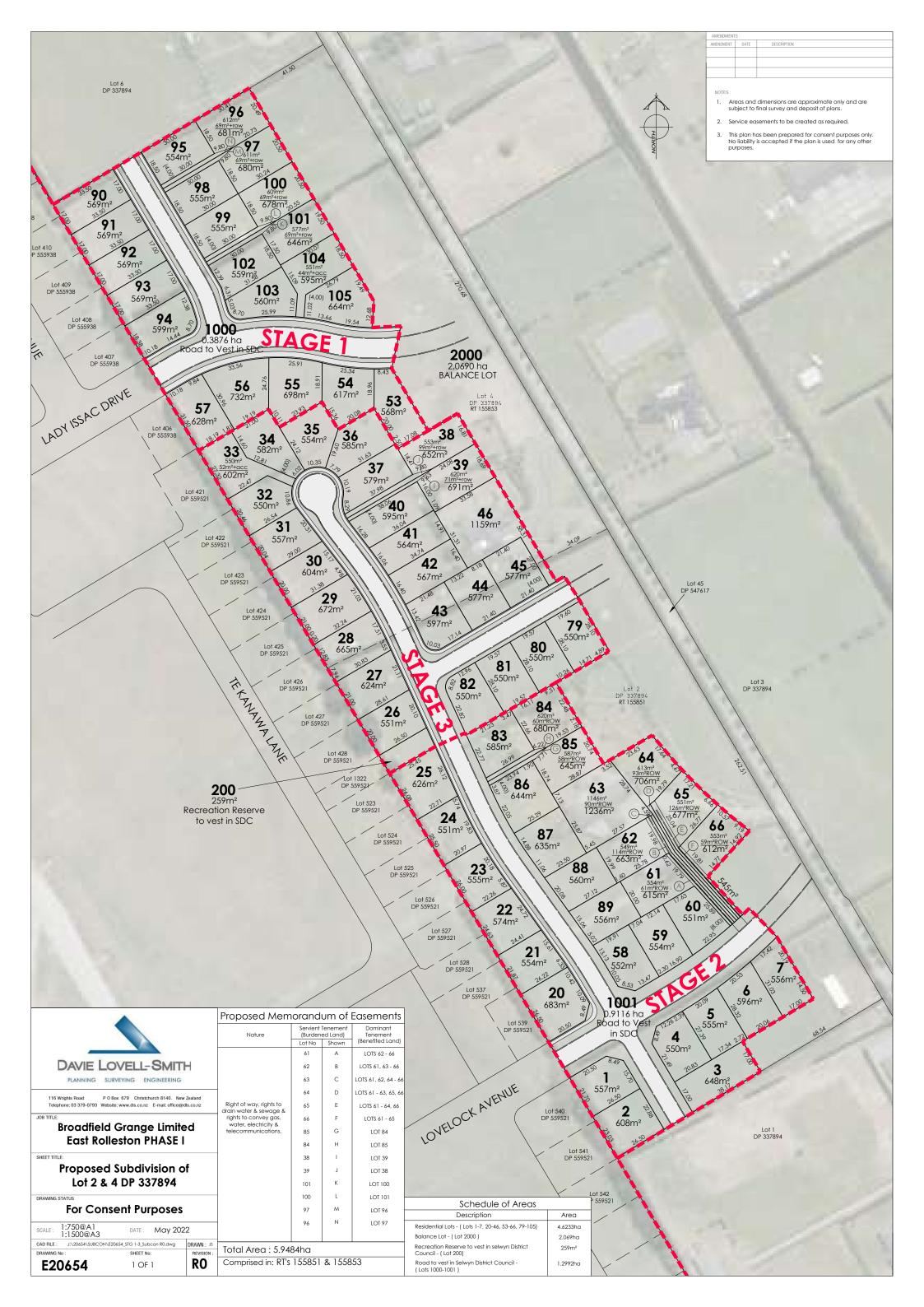




APPENDIX 1:

Development Plan



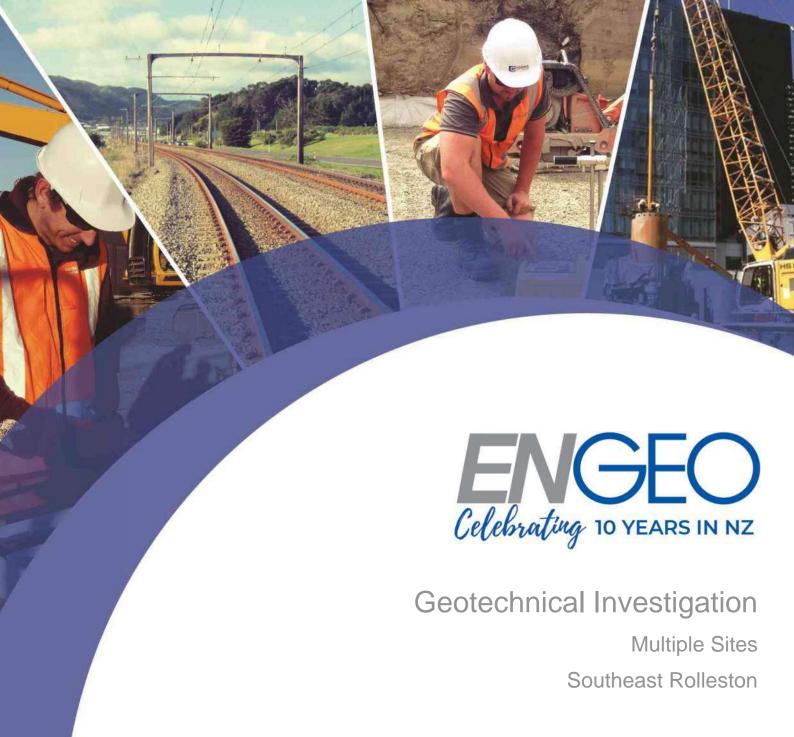




APPENDIX 2:

ENGEO Southeast Rolleston Geotechnical Investigation





Submitted to:

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Contents

1	Inti	troduction	1
2	Sit	te Description	1
3	Ge	eological Model	2
	3.1	Regional Geology	2
	3.2	Geomorphology	2
	3.3	Geohazards	2
		3.3.1 Seismicity	2
		3.3.2 Liquefaction and Lateral Spreading	3
	3.4	Flooding	3
	3.5	ECan Boreholes	4
	3.6	Site Seismic Class	6
4	Sit	te Investigation	6
	4.1	Site Investigation	6
5	Ge	eotechnical Assessment	6
	5.1	Site Seismic Class	6
	5.2	Liquefaction Assessment	6
	5.3	Foundations	7
6	RM	MA Section 106 Assessment	7
7	Re	eferences	8
8	l in	mitations	ç

Tables

Table 1: Generalised Summary of ECan Boreholes

Table 2: Summary of Subsurface Investigations

Figures

Figure 1: SDC Flooding map

Figure 2: Nearby ECan Borehole Locations



Appendices

Appendix 1: Site Plan

Appendix 2: TP Logs

Appendix 3: Ecan Boreholes

ENGEO Document Control:

Report Title	Geotechnical Investigation - Multip	le Sites, Southeas	t Rolleston	
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Distribution (PDF)	Justin McDonald, Brad Wilson (Urb	oan Estates)		
Date	Revision Details/Status	WP	Author	Reviewer
09/12/2020	Issued to Client	DF	JRW	NC



1 Introduction

ENGEO Ltd was requested by Urban Estates Ltd to undertake a geotechnical investigation of several properties (herein referred to as 'the site'). This work has been carried out in accordance with our signed agreement dated 18 August 2020.

We understand that you propose to apply for a plan change for this site to allow proposed zone change from general rural zone to general residential zone with an approximate density of 12 lots per hectare. Our scope of works at this stage will support your Resource Consent application for the plan change only. We can complete additional testing to support a subdivision consent application at a later date.

Our scope of works included the following:

- Review of published geotechnical and geological information relevant to the site;
- Site assessment by an experienced ground engineering professional;
- Coordinate local buried services location contractor;
- Shallow subsurface testing, consisting of approximately 0.3 tests (test pits) per hectare*, with a total of approximately 18 test pits; These pits will be up to approximately 2 m deep, 3 m long and 1 m wide. We will loosely backfill the test pits upon completion with the excavated soil. Re-compaction will be accomplished by tamping with the excavator bucket.
- Assess the liquefaction potential for the site based on our site investigations and published literature;
- Prepare a report outlining our findings on the ground conditions and the suitability of the site for residential subdivision. This will include:
 - o Foundation recommendations for typical timber framed residential dwellings.
 - Seismic Subsoil category;
 - Address likely geohazards that may affect the site; and
 - Provide general geotechnical recommendations related to the proposed development.

Our scope of works does not include geotechnical recommendations to a level suitable for subdivision consent, foundation design or Building Consent.

2 Site Description

The 63 ha site is located on a relatively flat area in Rolleston and made up of the following addresses (Appendix 1):

- 127 Lincoln Rolleston Road
- 391 Lincoln Rolleston Road
- 548 Selwyn Road



- 2/554 Selwyn Road
- 3/554 Selwyn Road
- 1/554 Selwyn Road
- 1/572 Selwyn Road (Lot 2 DP 337894)
- 2/572 Selwyn Road (Lot 3 DP 337894)
- 4/572 Selwyn Road (Lot 4 DP 337894)
- 5/572 Selwyn Road (Lot 5 DP 337894)
- 6/572 Selwyn Road (Lot 6 DP 337894)
- 582 Selwyn Road (Lot 1 DP 337894)

The site is currently lifestyle blocks mostly used for light grazing. There are various existing dwellings and sheds on-site.

3 Geological Model

3.1 Regional Geology

The site has been regionally mapped by GNS (Forsyth et al., 2008) as being underlain by brownish grey river alluvium (Q2a).

3.2 Geomorphology

The site comprises relatively flat ground, with gentle undulations and depressions in some areas. As evident on aerial imagery (Canterbury Maps, 2020) and observed during our site walkover conducted on 2 December 2020, a number of areas of undulating and depressed ground can be attributed to paleo-channels, which traverse the site in a general northwest to southeast direction. Based on observations, sandy silt deposits with variable thickness are expected to have in-filled the paleo-channels where they have not remained as channel features.

3.3 Geohazards

3.3.1 Seismicity

There are no known or mapped faults in the immediate area of the site, however, the site may be at risk of ground shaking induced by movement of other faults.

The site is located between two recently discovered fault systems, the Greendale Fault and the Port Hills Fault, the ruptures of which initiated the ongoing Canterbury Earthquake Sequence (CES). The Greendale Fault has been mapped approximately 6 km northwest of the site and trends roughly east-west with a surface rupture of approximately 28 km (GNS, 2015), while the Port Hills Fault remains unmapped as the fault did not rupture the surface. Movement on the Port Hills Fault is believed to have occurred at a depth of 1 km to 2 km below the ground surface.



Large regional areas of faulting (GNS, 2015) namely the Ashley Fault, Porters Pass-Amberley Fault Zone, and the Hope and Alpine Faults, are further afield but present a high seismic hazard to the Christchurch area due to the anticipated size of earthquakes generated. The largest of these faults is the Alpine Fault, which has a return period of 250-300 years and is expected to produce a M8 earthquake. The last rupture on the Alpine Fault is believed to have occurred in 1717 (Pettinga et al., 2001).

3.3.2 Liquefaction and Lateral Spreading

The site is located in an area mapped where "damaging liquefaction is unlikely" (NZGD Map CGD5140, 2012), and a "zone of very low liquefaction potential" (GNS, 2006).

3.4 Flooding

The site is outside of any defined flood zones in the Selwyn District Council (SDC) Operative District Plan (SDC, 2015). The closest flood zone is the Lower Plains Flood Area which is approximately 4 km southeast of the site towards the Port Hills.

The Selwyn District Council have carried out computer-based flood modelling to predict the extent and depth of flooding that could happen during a one-in-200-year and a one-in-500-year flood. Based on this modelling, the water depth through the site may be up to 0.8 m deep in the existing channel feature (Figure 1) during the 500 year flood.

Figure 1: SDC Flooding map

From Canterbury Maps and Selwyn Council. Not to scale.



3.5 ECan Boreholes

We have reviewed deep ECan borehole logs located on the site (as shown in *Excluded from the average as an outlier.

Figure 2), and have reviewed the monitoring well data from the monitoring wells on the site (part of the ECan Water Level Monitoring Network).

The logs for the wells located on the site, or close to the site boundary indicate the underlying soil generally comprises of gravels which extend to the bottom of the drill holes with isolated clay and silt layers at depth. The groundwater levels recorded in these wells are an average of approximately 10.6 m depth. The details for the wells are provided in Appendix 3 (including available drill logs).

Table 1: Generalised Summary of ECan Boreholes

ECan Borehole	Total Depth (m)	Initial Water Level Below Ground Level (m)	Generalised Borelog as Logged by Driller
M36/4015	28	10.5	Gravel to 28.3 m depth.
M36/7850	42	12.2	Gravel to 42 m depth.
BX23_0262	42	7.3	Gravel to the maximum depth of testing with a 3 m thick clay layer from 26 m to 29 m depth.
M36_1816	13.67	7.64	No data available.
M36_2996	59.4	9.6	No data available.
M26_3018	65.7	9.32	Not logged up to 39 m depth. Gravel from 39 m to 65.7 m depth.
M36_3836	56.6	10.7	Gravel to the maximum depth of drilling.
M36_4433	30	9.7	Gravel to the maximum depth of testing with a 1 m thick clay layer from 26 m to 27 m depth.
M36_7975	37.5	10	Gravel to the maximum depth of testing with a 1 m thick silt layer from 25 m to 26 m depth.
M36_7976	36	10.2	Gravel to the maximum depth of drilling.
M36_8002	66	12.1	Gravel to the maximum depth of testing with 1 m thick clay layers from 26 m to 27 m depth and 32 m to 33 m.
M36_8009	36	11.8	Gravel to the maximum depth of drilling.



ECan Borehole	Total Depth (m)	Initial Water Level Below Ground Level (m)	Generalised Borelog as Logged by Driller
M36_8138	36	14.2	Gravel to the maximum depth of drilling.
M36_8299	90	20.4*	Gravel to the maximum depth of drilling with a 6 m layer of river sands with some clay from 8 m to 14 m depth.
M36_8300	42	13.4	Gravel to the maximum depth of drilling.
M36_8392	36	10	Gravel to the maximum depth of drilling
Average Grou	indwater depth	10.6	

^{*}Excluded from the average as an outlier.

Figure 2: Nearby ECan Borehole Locations



Aerial photograph sourced from Canterbury Maps. Not to scale.



3.6 Site Seismic Class

In accordance with NZS 1170.5:2004, Class D applies to this particular site, defining it as a 'deep soft soil site'.

4 Site Investigation

4.1 Site Investigation

Site investigations to assess the shallow subsurface material types and strength characteristics were undertaken by ENGEO on 3 and 4 December 2020. Eighteen test pits with associated Scala penetrometer tests were completed to a maximum depth of 2.2 m below ground level.

The investigations revealed subsurface conditions across the site are consistent with the published geological mapping, as summarised in Table 2. Hand auger and test pit logs are attached as Appendix 2 of this report.

Table 2: Summary of Subsurface Investigations

Soil Type	Depth to Top of Layer (m)	General Layer Thickness (m)	Density / Consistency	Additional Comments
TOPSOIL	0.0	0.3	-	
SILT / SAND*	0.3	0.5**	Very Stiff to Hard / Medium Dense	-
Sandy GRAVEL	0.3	Unknown	Medium Dense to Very Dense	Tightly packed and consistent across the site. Deep roots observed up to 2 m depth.

^{*}Not observed in all test pit locations.

5 Geotechnical Assessment

5.1 Site Seismic Class

For the purpose of seismic design, we consider the soil classification in line with NZS 1170.5:2004 to be 'Class D – Deep or soft soil sites'

5.2 Liquefaction Assessment

Owing to the nature of the subsurface materials and depth to groundwater at the site, we consider the potential for liquefaction and lateral spreading on the site to be very low.

We therefore consider future land performance to be in line with Technical Category 1 (TC1), whereby future land damage from liquefaction is unlikely, and ground settlements are expected to be within normally accepted tolerances.



^{**}Thickness varies.

5.3 Foundations

Foundations for future one or two storey residential dwellings within the subdivision are likely to comprise pad, strip or slab foundations designed in accordance with the provisions of NZS 3604 Timber Framed Buildings. In areas where native gravel is < 400 mm of the surface, it is likely that standard 3604 foundations will be suitable. In areas where foundations are native on alluvial silt or sand, standard foundations will likely be suitable however engineering judgement may be required to design the foundations to accommodate for a reduced bearing capacity. This should be confirmed by lot specific reporting completed during the building consent process.

6 RMA Section 106 Assessment

Section 106 of the Resource Management Act 1991 states a consent authority may refuse to grant subdivision consent, or may grant a consent subject to specific consent conditions if the land is likely to be subject to the following:

- Erosion, including surface and subsurface erosion, associated with water and wind;
- Falling debris, including rockfall that could impact the site from upslope sources;
- Subsidence, which involves the removal of underlying support by natural or artificial means;
- Slippage, which is defined as the downslope transfer of materials by sliding and / or flowage;
 and
- Inundation, which may be sourced from streams, coastal processes or excess precipitation.

Based on our observations and the nature of the site, and the site's distance from the nearest significant watercourse, we consider it unlikely for the site to be subject to any of the above hazards and, as such, the site is considered suitable for a plan change from a geotechnical perspective. As discussed in Section 3.3 of this report, we recommend that flooding hazard is considered during subdivision design.



7 References

Canterbury Maps, Groundwater. Retrieved December 2020 from http://canterburymaps.govt.nz/Viewer.

Forsyth, P., Barrell, D. J., & Jongens, R. (2008). Sheet 16 - Geology of the Christchurch Area 1:250,000. Lower Hutt: Institute of Geological and Nuclear Sciences.

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The Ministry of Business, Innovation, and Employment. (2012). Guidance-Repairing and rebuilding houses affected by the Canterbury earthquakes. Christchurch: The Ministry of Business, Innovation, and Employment.

We also acknowledge the New Zealand GeoNet project and its sponsors EQC, GNS Science and LINZ, for providing data used in this report.



8 Limitations

- i. We have prepared this report in accordance with the brief as provided. This report has been prepared for the use of our client, Urban Estates Ltd, their professional advisers and the relevant Territorial Authorities in relation to the specified project brief described in this report. No liability is accepted for the use of any part of the report for any other purpose or by any other person or entity.
- ii. The recommendations in this report are based on the ground conditions indicated from published sources, site assessments and subsurface investigations described in this report based on accepted normal methods of site investigations. Only a limited amount of information has been collected to meet the specific financial and technical requirements of the client's brief and this report does not purport to completely describe all the site characteristics and properties. The nature and continuity of the ground between test locations has been inferred using experience and judgement and it should be appreciated that actual conditions could vary from the assumed model.
- iii. Subsurface conditions relevant to construction works should be assessed by contractors who can make their own interpretation of the factual data provided. They should perform any additional tests as necessary for their own purposes.
- iv. This Limitation should be read in conjunction with the Engineering NZ/ACENZ Standard Terms of Engagement.
- v. This report is not to be reproduced either wholly or in part without our prior written permission.

We trust that this information meets your current requirements. Please do not hesitate to contact the undersigned on (03) 328 9012 if you require any further information.

Report prepared by

Jed Watts

Engineering Geologist

Report reviewed by

Neil Charters, CMEngNZ (CPEng)

Principal Geotechnical Engineer





APPENDIX 1:

Site Plan







APPENDIX 2:

TP Logs





Geotechnical Investigation 548-572 Selwyn Road Southwest Rolleston 18113.000.001

Client: Urban Estates Shear Vane No : Date : 3/12/2020

 $\textbf{Logged By}: \mathsf{DD/DKi}$ Reviewed By : JRW

Max Test Pit Depth : 2 m Digger Type/Size : Bucket Excavator / 5 tonne

Latitude: -43.611992 **Longitude**: 172.408565

		181	13.0	UU.C	JU 1								∋ : 172.	40856	5	
Material	Easier)	cavata lative S	y (elso Harder Harder	USCS Symbol	DESC	CRIPTION	Graphic Symbol	Elevation (mRL)	Water Level	Moisture Cond.	Consistency/ Density Index	Shear Vane Peak/Remolded (kPa)	Blo	ws per	100mr	m
TOPSOIL				ML	brown. Low plastic	citv. Sand is fine to	7. 3.7. 7. 3.7. 7. 3.7.			D	N/A					
		·			cobbles and trace Well graded. Grav subrounded. Sand graded.	rootlets; greyish brown el subangular to I fine to coarse, well					D					<i>//</i>
ALLUVIUM				GW						М	Tightly Packed					
-					Depth of Excavation Termination Cond	on: 2 m ition: Target depth										
													<u> </u>			-
	TOPSOIL	TOPSOIL Material Easier ®	TOPSOIL Material (Relative S	TOPSOIL Material (Kelative Scale) Harder Harder	TOPSOIL Material Excavatapility (Kelative Scale) Harder Harder WT	Excavatability (Relative Scale) Barriage Part Part	Excavatability (Relative Scale) Fig. September September	Excavatability (Relative Scale) DESCRIPTION DESCRIPT	Excavatability (Relative Scale) Fig. Pare P	Excavatability (Relative Scale) The part of the pa	Excavatability (Relative Scale) DESCRIPTION DESCRIPTI	Excavatability (Relative Scale) Pour land Pour l	Excavatability (Relative Scale) Odd (Rela	Excavatability (Relative Scale) Part Pa	Excavatability (Relative Scale) DESCRIPTION Descriptio	Excavatability (Relative Scale) Relative Scale



Geotechnical Investigation 548-572 Selwyn Road Southwest Rolleston 18113.000.001

Client: Urban Estates Shear Vane No :

Date : 3/12/2020 $\textbf{Logged By}: \mathsf{DD/DKi}$ Max Test Pit Depth : 2 m Reviewed By : JRW

Digger Type/Size : Bucket Excavator / 5 tonne Latitude: -43.610429 Longitude: 172.408787 Bucket Type/Size: Toothed / 400 mm

					Bucket Type/Size	loothed /	/ 400 i	mm			Longitude	e: 1/2.	4087	87		
Depth (m BGL)	Material	Excavatability (Relative Scale) Jack Harder Harder H	USCS Symbol		CRIPTION	Graphic Symbol	Elevation (mRL)	Water Level	Moisture Cond.	Consistency/ Density Index	Shear Vane Peak/Remolded (kPa)	Blo		netron er 100 8 1		
- C	IOPSOIL		ML	Sandy SILT with tr brown. Low plastic medium [TOPSOIL	ace gravel and rootlets; ity. Sand is fine to _].	17.17.17 17.17.17 17.17.17				N/A		•		•		
0.5 -			ML	SILT with minor sa brown with orange Low plasticity.	and and rootlets; light mottles. Sand is fine.				D	H VSt			<i></i>		•	
				cobbles; greyish br	to subrounded. Sand	X				D				\		>:
1.0	ALLUVIUM		GW							Tightly Packed						
1.5 -									M							
2.0				Depth of Excavation Termination Condi	on: 2 m tion: Target depth				W							
Scala	Pe	net target depth enetrometer met groundwater w	pract	ical refusal at 0.6 m	depth.	_				_		_				



Geotechnical Investigation 548-572 Selwyn Road Southwest Rolleston 18113.000.001

Client: Urban Estates Shear Vane No :

Date : 3/12/2020 $\textbf{Logged By}: \mathsf{DD/DKi}$ Max Test Pit Depth : 2.2 m Reviewed By : JRW

Digger Type/Size : Bucket Excavator / 5 tonne **Latitude**: -43.613335 Longitude: 172.408111 Bucket Type/Size: Toothed / 400 mm

Excavatability (Relative Scale)	M M Water Symbol	Sandy SILT with to brown. Low plastic medium [TOPSOI	crace gravel and rootlets; city. Sand is fine to L.]. The GRAVEL with minor ets; greyish brown. Well bangular to subrounded. well graded. observed from 0.55 m.	Graphic Symbol	Elevation (mRL)	Water Level	о Moisture Cond.	Consistency/ Density Index	Shear Vane Peak/Remolded (kPa)	Blo		er 10	00mm 10 12
		Sandy fine to coar	L]. se GRAVEL with minor				D	N/A					
	GW	cobbles and rootle	ts grevish brown Well					<u>D</u>					Α
	GW												
								Tightly Packed					
							M						
		Double of Free web.											
		Termination Cond	on: 2.2 m ition: Target depth										
m	et target depth	et target depth at 2.2	Depth of Excavation Cond Termination Cond et target depth at 2.2 m. letrometer met practical refusal at 0.4 m	Depth of Excavation: 2.2 m Termination Condition: Target depth et target depth at 2.2 m. letrometer met practical refusal at 0.4 m depth.	Depth of Excavation: 2.2 m Termination Condition: Target depth et target depth at 2.2 m. let target depth at 2.2 m. let rometer met practical refusal at 0.4 m depth.	Depth of Excavation: 2.2 m Termination Condition: Target depth et target depth at 2.2 m. let target depth at 2.2 m. let target met practical refusal at 0.4 m depth.	Depth of Excavation: 2.2 m Termination Condition: Target depth et target depth at 2.2 m. let target depth at 2.2 m. let target met practical refusal at 0.4 m depth.	Depth of Excavation: 2.2 m Termination Condition: Target depth et target depth at 2.2 m. let target depth at 2.2 m. let rometer met practical refusal at 0.4 m depth.	Depth of Excavation: 2.2 m Termination Condition: Target depth et target depth at 2.2 m. let target depth at 2.2 m. let rometer met practical refusal at 0.4 m depth.	Depth of Excavation: 2.2 m Termination Condition: Target depth	Depth of Excavation: 2.2 m Termination Condition: Target depth et target depth at 2.2 m. let target depth at 2.2 m. let target depth at 2.4 m depth.	Depth of Excavation: 2.2 m Termination Condition: Target depth et target depth at 2.2 m. let target depth at 2.2 m. let target depth at 2.4 m depth.	Depth of Excavation: 2.2 m Termination Condition: Target depth



Geotechnical Investigation 548-572 Selwyn Road Southwest Rolleston 18113.000.001

Client: Urban Estates Shear Vane No : Date : 3/12/2020

 $\textbf{Logged By}: \mathsf{DD/DKi}$ Reviewed By : JRW

Max Test Pit Depth : 2 m Digger Type/Size : Bucket Excavator / 5 tonne Bucket Type/Size: Toothed / 400 mm

Latitude : -43.613667 **Longitude**: 172.411393

							Bucket Type/Size	lootnea	/ 400	mm			Longitud	# : 1/2.	411393		
Depth (m BGL)	Material	Excav (Relativ	atabili ve Sca	Harder ele kti	USCS Symbol	DESC	CRIPTION	Graphic Symbol	Elevation (mRL)	Water Level	Moisture Cond.	Consistency/ Density Index	Shear Vane Peak/Remolded (kPa)		a Penetovs per 1	00mn	n
-	OPSOIL				ML	Sandy SILT with to brown. Low plastic medium [TOPSOI	race gravel and rootlets; city. Sand is fine to L].	17 · 77 · 17				N/A		•			
	Ĕ					SILT with minor so brown with orange medium. Low plas	and and rootlets; light mottles. Sand is fine to ticity.					VSt			·		
0.5					ML						D	н					
1.0-	V			-		cobbles and trace Well graded. Grav	se GRAVEL with minor rootlets; greyish brown. el subangular to l fine to coarse, well										λ
-	ALLUVIUM				GW	Rootlets were not	observed from 1.1 m.					Tightly Packed					
1.5 -											М						
2.0			;			Depth of Excavation Termination Cond	on: 2 m ition: Target depth	6 6									
-																	
														· :	· · ·		<u>.</u>
Scala	a Pe	met targ	neter i	met	practi	n. cal refusal at 0.6 m	ı depth.										



Geotechnical Investigation 548-572 Selwyn Road Southwest Rolleston 18113.000.001

Client: Urban Estates Shear Vane No : Date : 3/12/2020

 $\textbf{Logged By}: \mathsf{DD/DKi}$ Reviewed By : JRW

Digger Type/Size : Bucket Excavator / 5 tonne Latitude : -43.614262 Longitude: 172.410054 Bucket Type/Size : Toothed / 400 mm

						Bucket Type/Size		, 1001				. 5	: 172.410054			
Depth (m BGL) Material Easier Basier		cavatability ative Scale) Japan EH		DES	CRIPTION	Graphic Symbol	Elevation (mRL)	Water Level	Moisture Cond.	Consistency/ Density Index	Shear Vane Peak/Remolded (kPa)	Scala Penetrometer Blows per 100mm				
	TOPSOIL			ML	Sandy SILT with t brown. Low plasti medium [TOPSOI	race gravel and rootlets city. Sand is fine to L].		Ш	S	D	N/A	ш	2	4	6 8	10 12
0.5 - - - -	-				Sandy fine to coal cobbles; greyish b Gravel subangula fine to coarse, we	rse GRAVEL with mino rown. Well graded. r to subrounded. Sand ll graded.										
- 1.0— - - -	ALLUVIUM			GW	Rootlets were not	observed from 1.2 m.				М	Tightly Packed					
1.5 - - - - -	V				Gravel becomes f m depth.	ine to medium from 1.7										
2.0— - - -					Depth of Excavati Termination Cond	on: 2.2 m ition: Target depth										
														•	•	, ,

Max Test Pit Depth : 2.2 m



Geotechnical Investigation 548-572 Selwyn Road Southwest Rolleston 18113.000.001

Client: Urban Estates Shear Vane No :

Date : 3/12/2020 $\textbf{Logged By}: \mathsf{DD/DKi}$ Max Test Pit Depth : 2.2 m Reviewed By : JRW

Digger Type/Size : Bucket Excavator / 5 tonne **Latitude**: -43.616713 Bucket Type/Size : Toothed / 400 mm **Longitude**: 172.411568

Depth (m BGL)	Material	Excav (Relating) ise ise ise	atability ve Scal	symbo	DESCRIPTION	Graphic Symbol	Elevation (mRL)	Water Level	Moisture Cond.	Consistency/ Density Index	Shear Vane Peak/Remolded (kPa)	Scala Penetrome Blows per 100m 2 4 6 8 10
-	TOPSOIL			ML	Sandy SILT with trace gravel and rootle brown. Low plasticity. Sand is fine to medium [TOPSOIL].	ts; (1/2) 1/2 1/2 1/2 1/2 1/2 1/2 1/2 1/2 1/2 1/2				N/A		•
- 0.5 - - -	-				Sandy fine to coarse GRAVEL with min cobbles and trace rootlets; greyish brow Well graded. Gravel subangular to subrounded. Sand fine to coarse, well graded.	or /n.			D	D		•
- - 1.0- - -	MUIV				No rootlets from 0.8 m to 1.6 m.							
- 1.5 - - -	ALLUVIUM			GW	Minor rootlets encountered from 1.6 m depth.				М	Tightly Packed		
2.0— - -	-				Depth of Excavation: 2.2 m Termination Condition: Target depth							
					Tommation Condition. Target depart							



Geotechnical Investigation 548-572 Selwyn Road Southwest Rolleston 18113.000.001 Client: Urban Estates

Shear Vane No :

Date: 3/12/2020 Max Test Pit Depth: 2 m Logged By: DD/DKi Reviewed By: JRW

Digger Type/Size : Bucket Excavator / 5 tonne Bucket Type/Size : Toothed / 400 mm **Latitude**: -43.617708 **Longitude**: 172.41176

				Buoket Typeroize : 14							
Depth (m BGL)	Material	Excavatability (Relative Scale) Harder	USCS Symbol	DESCRIPTION	Graphic Symbol	Elevation (mRL)	Water Level	Moisture Cond.	Consistency/ Density Index	Shear Vane Peak/Remolded (kPa)	Scala Penetrometer Blows per 100mm 2 4 6 8 10 12
_	ropsoil		ML	Sandy SILT with trace gravel and rootlets; brown. Low plasticity. Sand is fine to medium [TOPSOIL].	\(\frac{1}{2}\), \(\frac{1}\), \(\frac{1}2\), \(\frac{1}2\), \(\frac{1}2\), \(\frac{1}2\), \(\frac{1}2\), \(N/A		•
0.5 -	F		ML	SILT with some sand and trace rootlets; light brown. Sand is fine to medium. Low plasticity.				D	F St - VSt		
1.0— - 1.5 2.0—	ALLUVIUM		GW	Sandy fine to coarse GRAVEL with minor cobbles and trace rootlets; greyish brown. Well graded. Gravel subangular to subrounded. Sand fine to coarse, well graded. Rootlets were not observed from 0.75 m.				M	Tightly Packed		<u> </u>
2.0 - - -				Termination Condition: Target depth							

Test pit met target depth at 2 m.

Scala Penetrometer met practical refusal at 0.7 m depth.

Standing groundwater was not encountered



Geotechnical Investigation 548-572 Selwyn Road Southwest Rolleston 18113.000.001

Client: Urban Estates

Shear Vane No :

Date : 3/12/2020 Max Test Pit Depth : 2.1 m

 $\textbf{Logged By}: \mathsf{DD/DKi}$ Reviewed By : JRW

Digger Type/Size : Bucket Excavator / 5 tonne Bucket Type/Size : Toothed / 400 mm

Latitude: -43.61702 **Longitude**: 172.414053

						Bucket Type/Size	ootineu	4001				Longitude	. 17	2.4	1400	5	
Depth (m BGL)	Material	Easier (Relati	vatab ive So	vility cale) Harder	USCS Symbol	DESCRIPTION	Graphic Symbol	Elevation (mRL)	Water Level	Moisture Cond.	Consistency/ Density Index	Shear Vane Peak/Remolded (kPa)			s per	100	meter mm
-	TOPSOIL				ML	Sandy SILT with minor gravel and rootlets; brown. Low plasticity. Sand is fine to medium [TOPSOIL].	$\frac{7}{7}, \frac{7}{7}, \frac{7}{7}$				N/A			•			
0.5 -	 					Sandy fine to coarse GRAVEL with minor cobbles and trace rootlets; greyish brown. Well graded. Gravel subangular to subrounded. Sand fine to coarse, well graded.	XXX.			D							
- 1.0- - -	ALLUVIUM				GW	Rootlets were not observed from 0.75 m.					Tightly Packed						
- 1.5 - - -										М							
2.0							X										
-						Depth of Excavation: 2.1 m Termination Condition: Target depth											
Sca	la Pe	met tai	neter	met	pract	I m. ical refusal at 0.2 m depth. t encountered											



Geotechnical Investigation 548-572 Selwyn Road Southwest Rolleston 18113.000.001

Client: Urban Estates Date : 3/12/2020

Shear Vane No :

 $\textbf{Logged By}: \mathsf{DD/DKi}$ Reviewed By : JRW

Digger Type/Size : Bucket Excavator / 5 tonne Bucket Type/Size : Toothed / 400 mm

Max Test Pit Depth : 2.1 m

Latitude: -43.611618 **Longitude**: 172.410134

					Bucket Type/Size	oothed	/ 400 i	mm			Longitude	3:1/2	.410	134	
Depth (m BGL)	Material	Excavatability (Relative Scale	ymbc	DES	CRIPTION	Graphic Symbol	Elevation (mRL)	Water Level	Moisture Cond.	Consistency/ Density Index	Shear Vane Peak/Remolded (kPa)	Blo		er 10	ometer 00mm 10 12
-	OPSOIL		ML	Sandy SILT with t brown. Low plastic medium [TOPSOI	race gravel and rootlets; city. Sand is fine to L].	17 · 31 · 17				N/A		:	•		
0.5 -	Τ		ML	SILT with minor s brown with orange medium. Low plas	and and rootlets; light e mottles. Sand is fine to sticity.				D	VSt - H			4	•	
1.0-	VIUM			cobbles and trace Well graded. Grav	rse GRAVEL with minor rootlets; greyish brown. /el subangular to d fine to coarse, well										
1.5 -	ALLUVIUM		GW	Rootlets were not	observed from 1.3 m.				М	Tightly Packed					
2.0				Depth of Excavation Termination Cond	on: 2.1 m ition: Target depth										
Test Scal	pit ı	met target dept	h at 2.	1 m. tical refusal at 0.4 n	n depth.										

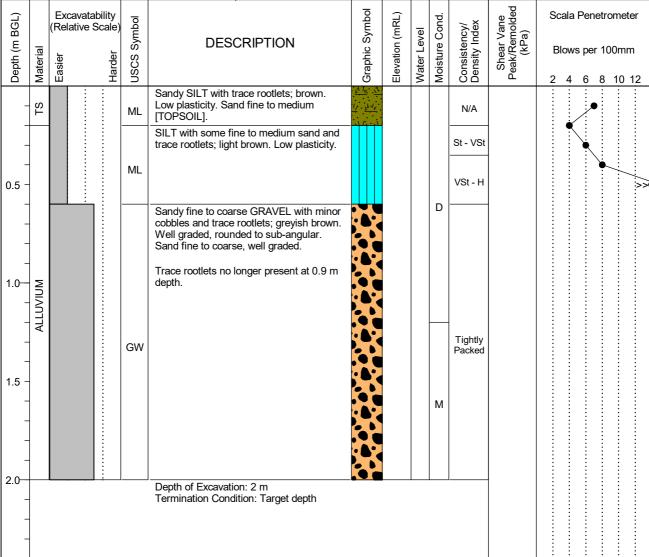


Geotechnical Investigation 548-572 Selwyn Road Southwest Rolleston 18113.000.001

Shear Vane No: N/A Client: Urban Estates Date: 03/12/2020 Logged By : DD/DKi Max Test Pit Depth: 2 m Reviewed By: JRW

Digger Type/Size : Bucket Excavator / 5 tonne Latitude: -43.614022 Longitude: 172.411991 Bucket Type/Size : Toothed / 400 mm

Scala Penetrometer



Test pit reached target depth at 2 m. Scala Penetrometer met practical refusal at 0.5 m depth. Standing groundwater was not encountered

GEOTECH TEST PIT LOG ROLLESTON TP 10 TO 18.GPJ NZ MASTER DATA TEMPLATE.GDT 7/12/20



Geotechnical Investigation 548-572 Selwyn Road Southwest Rolleston 18113.000.001

Digger Type/Size: Bucket Excavator / 5 tonne Bucket Type/Size: Toothed / 400 mm

Shear Vane No: N/A Logged By: DD/DKi Reviewed By: JRW Latitude: -43.612378

Longitude: 172.413979

Shear Vane Peak/Remolded (kPa) Excavatability Graphic Symbol Scala Penetrometer Depth (m BGL) Symbol Elevation (mRL) Moisture Cond. Consistency/ Density Index (Relative Scale) Water Level **DESCRIPTION** Material Blows per 100mm Harder JSCS Easier 6 8 10 12 Sandy SILT with trace rootlets; brown. Low plasticity. Sand fine to medium [TOPSOIL]. 2 N/A ML SILT with some fine to medium sand and trace rootlets; light brown. Low plasticity. St - VSt Μ 0.5 ML Sandy fine to coarse GRAVEL with minor D cobbles and trace rootlets; greyish brown.
Well graded, rounded to sub-angular. Sand fine to coarse, well graded. 1.0-Trace rootlets no longer present at 1.0 m depth. GW Tightly Packed W 1.5 Depth of Excavation: 2 m Termination Condition: Target depth

Test pit reached target depth at 2 m. Scala Penetrometer met practical refusal at 0.8 m depth. Standing groundwater was not encountered

GEOTECH TEST PIT LOG ROLLESTON TP 10 TO 18.GPJ NZ MASTER DATA TEMPLATE.GDT 7/12/20



Geotechnical Investigation 548-572 Selwyn Road Southwest Rolleston 18113.000.001

Client: Urban Estates Shear Vane No: N/A Date : 03/12/2020 Logged By : DD/DKi Max Test Pit Depth : 2 m Reviewed By : JRW

Digger Type/Size : Bucket Excavator / 5 tonne Latitude : -43.615236 Bucket Type/Size : Toothed / 400 mm **Longitude**: 172.412995

Sandy fine to coarse GRAVEL with minor cobbles and trace rootlets; greyish brown. Well graded, rounded to sub-angular. Sand fine to coarse, well graded. GW Trace rootlets no longer present at 1.4 m depth. Tightly Packed	Sandy SILT with trace rootlets; brown. Low plasticity. Sand fine to medium [TOPSOIL]. SILT with some fine to medium sand and trace rootlets; light brown. Low plasticity. Sandy fine to coarse GRAVEL with minor cobbles and trace rootlets; greyish brown. Well graded, rounded to sub-angular. Sand fine to coarse, well graded. Tightly Packed Trace rootlets no longer present at 1.4 m depth. M Depth of Excavation: 2 m	3GL)		Excavatal	sility		·	_					9		
Sandy SILT with trace rootlets; brown. Low plasticity. Sand fine to medium [TOPSOIL]. SILT with some fine to medium sand and trace rootlets; light brown. Low plasticity. Sandy fine to coarse GRAVEL with minor cobbles and trace rootlets; greyish brown. Well graded, rounded to sub-angular. Sand fine to coarse, well graded. Tightly Packed Trace rootlets no longer present at 1.4 m depth. M Depth of Excavation: 2 m	Sandy SILT with trace rootlets; brown. Low plasticity. Sand fine to medium [TOPSOIL]. SILT with some fine to medium sand and trace rootlets; light brown. Low plasticity. Sandy fine to coarse GRAVEL with minor cobbles and trace rootlets; greyish brown. Well graded, rounded to sub-angular. Sand fine to coarse, well graded. Tightly Packed Trace rootlets no longer present at 1.4 m depth. M Depth of Excavation: 2 m	Depth (m E		Relative S	cale)	USCS Symbol	DESCRIPTION	Graphic Symbo	Elevation (mRL)	Water Level	Moisture Cond.	Consistency/ Density Index	Shear Vane Peak/Remolde (kPa)	Blows per 10	0mm
trace rootlets; light brown. Low plasticity. Sandy fine to coarse GRAVEL with minor cobbles and trace rootlets; greyish brown. Well graded, rounded to sub-angular. Sand fine to coarse, well graded. GW Trace rootlets no longer present at 1.4 m depth. Depth of Excavation: 2 m	trace rootlets; light brown. Low plasticity. D					ML	Low plasticity. Sand fine to medium	1/2 · 2/4 · 1/2 · 2/4 · 1/2 · 2/4 · 1/2 · 2/4 · 1/2 · 2/4 ·						•	
Sandy line to coarse GRACE With minor cobbles and trace rootlets; greyish brown. Well graded, rounded to sub-angular. Sand fine to coarse, well graded. GW Trace rootlets no longer present at 1.4 m depth. Depth of Excavation: 2 m	Sandy line to coarse GRAZE With minor cobbles and trace rootlets; greyish brown. Well graded, rounded to sub-angular. Sand fine to coarse, well graded. GW Trace rootlets no longer present at 1.4 m depth. Tightly Packed M Depth of Excavation: 2 m	0.5				ML	SILT with some fine to medium sand and trace rootlets; light brown. Low plasticity.					VSt - H			
2.0 Depth of Excavation: 2 m	2.0 Depth of Excavation: 2 m						cobbles and trace rootlets; greyish brown. Well graded, rounded to sub-angular.				D				
Depth of Excavation: 2 m	Depth of Excavation: 2 m	ALLUVIU	NEC NO.			GW	Trace rootlets no longer present at 1.4 m depth.				M	Tightly Packed			
		2.0					Depth of Excavation: 2 m Termination Condition: Target depth								



Geotechnical Investigation 548-572 Selwyn Road Southwest Rolleston 18113.000.001

Client: Urban Estates Shear Vane No: N/A Date: 03/12/2020 Max Test Pit Depth : 1.8 m

Digger Type/Size : Bucket Excavator / 5 tonne Bucket Type/Size : Toothed / 400 mm

Logged By : DD/DKi Reviewed By : JRW Latitude: -43.616271

Longitude: 172.416113

						Bucket Type/Size .	Toolified	/ 4 00 i				Longitude	. 17	2.4	101	10		
Depth (m BGL)	Material	Easier System	avata ative \$	tarder (aleog	USCS Symbol	DESCRIPTION	Graphic Symbol	Elevation (mRL)	Water Level	Moisture Cond.	Consistency/ Density Index	Shear Vane Peak/Remolded (kPa)			s pe	er 10	omet 00mr 10	m
_	TS				ML	Sandy SILT with trace gravel and rootlets brown. Low plasticity. Sand fine to medium [TOPSOIL].	; <u>\(\frac{1}{2} \cdot </u>				N/A		:		•	····/	/	·····/
0.5 -						Sandy fine to coarse GRAVEL with minor cobbles and trace rootlets; greyish brown Well graded, rounded to sub-angular. Sand fine to coarse, well graded.				D								Λ
1.0-	ALLUVIUM				GW	Trace rootlets no longer present at 0.85 n depth.				М	Tightly Packed							
1.5 -																		
- 2.0— - -						Depth of Excavation: 1.8 m Termination Condition: Target depth												
Sca	la Pe	enetro	omete	er met	practi	t 1.8 m. cal refusal at 0.2 m depth. encountered												_



Geotechnical Investigation 548-572 Selwyn Road Southwest Rolleston 18113.000.001

Client: Urban Estates Shear Vane No: N/A Date : 04/12/2020 $\textbf{Logged By}: \mathsf{DD/DKi}$ Max Test Pit Depth : 2 m Reviewed By : JRW Latitude: -43.610817

Digger Type/Size : Bucket Excavator / 5 tonne Longitude: 172.41651 Bucket Type/Size : Toothed / 400 mm

Depth (m BGL)	Material	Easier ()	cavatal ative S	tarder (aleog	USCS Symbol	DESCRIPTION	Graphic Symbol	Elevation (mRL)	Water Level	Moisture Cond.	Consistency/ Density Index	Shear Vane Peak/Remolded (kPa)				etrom	
- De	TOPSOIL	Ea		H _E	ML	Sandy SILT with trace rootlets; brown. Low plasticity. Sand fine to medium [TOPSOIL].	D 34 1/2	<u> </u>	W	M	N/A	<u> </u>	2	4	6	8 1	0 12
0.5					ML	SILT with some fine to medium sand and trace rootlets; light brown. Low plasticity.					VSt - H						
-					SW	Fine to coarse SAND with trace rootlets; brown. Well graded.					Loosely Packed						
1.0-	ALLUVIUM					Sandy fine to coarse GRAVEL with trace cobbles and rootlets; greyish brown. Well graded, rounded to sub-angular. Sand fine to coarse, well graded.				D							
1.5 -	-				GW	Trace rootlets no longer present at 1.6 m depth.					Loosely Packed						
AS IEK DAIA IEMPLAIE.GDI 7/12/20			<u> </u>	;		Depth of Excavation: 2 m Termination Condition: Target depth											
GEOTECH TEST PIT LOG KOLLESTON IP 10 10 18 GPJ N. MASTEK DATA II SS SE SS SE																	
Tes Sca Sta	ala P	eneti	hed tai romete oundwa	r met	practi	at 2 m. ical refusal at 0.3 m depth. t encountered											



Geotechnical Investigation 548-572 Selwyn Road Southwest Rolleston 18113.000.001

Client: Urban Estates Shear Vane No: N/A Date : 04/12/2020 $\textbf{Logged By}: \mathsf{DD/DKi}$ Max Test Pit Depth : 2 m Reviewed By : JRW

Digger Type/Size : Bucket Excavator / 5 tonne **Latitude**: -43.611312 Bucket Type/Size : Toothed / 400 mm Longitude: 172.413467

						Bucket Type/Size	ooti ieu /	400	111111			Longitude	. 17	2.41	3407		
Depth (m BGL)	Material	Easier le S	cavata ative S	y (elbos Harder Harder	USCS Symbol	DESCRIPTION	Graphic Symbol	Elevation (mRL)	Water Level	Moisture Cond.	Consistency/ Density Index	Shear Vane Peak/Remolded (kPa)		lows	per 1	omete 00mn	n
_	TOPSOIL				ML	Sandy SILT with trace rootlets; brown. Low plasticity. Sand fine to medium [TOPSOIL].					N/A			•		•	
0.5 -					ML	SILT with some fine to medium sand and trace rootlets; light brown. Low plasticity.				D	Н					•	
-			·			Sandy fine to coarse GRAVEL with minor cobbles, trace silt and rootlets; greyish brown. Well graded, rounded to sub-angular. Sand fine to coarse, well graded.											
1.0	ALLUVIUM				GW	Trace rootlets no longer present at 1.0 m depth.				М	Tightly Packed						
- 1.5 - -											racked						
- 2.0—						Depth of Excavation: 2 m				W							
_						Termination Condition: Target depth											
													<u>;</u>	-:	: :	;	-
Sca	la Pe	enetr	hed ta omete undwa	er met	practi	at 2 m. ical refusal at 0.5 m depth. encountered											



Geotechnical Investigation 548-572 Selwyn Road Southwest Rolleston 18113.000.001

Client: Urban Estates Shear Vane No: N/A Date: 04/12/2020 $\textbf{Logged By}: \mathsf{DD/DKi}$ Max Test Pit Depth : 2.2 m Reviewed By : JRW

Digger Type/Size : Bucket Excavator / 5 tonne Latitude: -43.613861 Bucket Type/Size : Toothed / 400 mm **Longitude**: 172.415625

						Bucket Type/Size	oou ica	1001					3 . 17 Z.4 130Z3
Depth (m BGL)	Material	Exc Rela (Re) (Rela (Rela (Re) (Rela (Re) (Rela (Re) (Re) (Re) (Re) (Re) (Re) (Re) (Re)	avatak ative S	oility scale) Harder H	USCS Symbol	DESCRIPTION	Graphic Symbol	Elevation (mRL)	Water Level	Moisture Cond.	Consistency/ Density Index	Shear Vane Peak/Remolded (kPa)	Scala Penetromete Blows per 100mm 2 4 6 8 10 1
-	TOPSOIL				ML	Sandy SILT with trace rootlets; dark brown. Low plasticity. Sand fine to medium [TOPSOIL].	17 · 27 · 7 · 7 · 7 · 7 · 7 · 7 · 7 · 7 ·				N/A)
-					ML	SILT with some fine to medium sand and trace rootlets; light brown. Low plasticity.					VSt - H		
0.5 - - - -						Sandy fine to coarse GRAVEL with minor cobbles and trace rootlets; greyish brown. Well graded, rounded to sub-angular. Sand fine to coarse, well graded.	TO THE PROPERTY OF THE PROPERT				D		•
1.0 - -	ALLUVIUM				GW	Trace rootlets no longer present at 1.3 m depth.				M	Tightly		
1.5 -											Packed		
_						Double of Francisco CO as	t			W			
-						Depth of Excavation: 2.2 m Termination Condition: Target depth							
Tes Sca	t pit i	reach enetre	omete	r met	pract	at 2.2 m. ical refusal at 0.6 m depth. t encountered							



Geotechnical Investigation 548-572 Selwyn Road Southwest Rolleston 18113.000.001

Client: Urban Estates Shear Vane No: N/A Date: 04/12/2020 Logged By : DD/DKi Max Test Pit Depth : 2.1 m Reviewed By : JRW

Digger Type/Size : Bucket Excavator / 5 tonne Latitude : -43.612026 Longitude: 172.417754 Bucket Type/Size : Toothed / 400 mm

							Bucket Type/Size :	Loothed	/ 400	mm			Longitude	3:1/2	2.41//	54	
Depth (m BGL)	Material	Excav (Relati	vatabil ve Sc	Harder (alg	USCS Symbol	DESC	CRIPTION	Graphic Symbol	Elevation (mRL)	Water Level	Moisture Cond.	Consistency/ Density Index	Shear Vane Peak/Remolded (kPa)	Blo		netron er 100 8 1	
-	TOPSOIL				ML	with dark brown m	race rootlets; light browr nottles. Low plasticity. um [TOPSOIL/FILL].	1				N/A		•			
0.5					ML		ne to medium sand and t brown. Low plasticity.					VSt - H			•		
					SP		AND with some silt and the brown with orange aded.					MD			•		
1.0-	ALLUVIUM			•		Sandy fine to coar cobbles; greyish b rounded to sub-an coarse, well grade	se GRAVEL with minor rown. Well graded, gular. Sand fine to d.				M	D					/ .^
1.5 -					GW							Tightly Packed					
2.0—											W						
-			-, · · · ·			Depth of Excavation Termination Cond	on: 2.1 m ition: Target depth										
Scal	la Pe	enetror	neter	met	practi	at 2.1 m. ical refusal at 1 m c	depth.										



Geotechnical Investigation 548-572 Selwyn Road Southwest Rolleston 18113.000.001

Client: Urban Estates Shear Vane No: N/A Date : 04/12/2020 $\textbf{Logged By}: \mathsf{DD/DKi}$ Max Test Pit Depth : 2 m Reviewed By : JRW

Digger Type/Size : Bucket Excavator / 5 tonne Bucket Type/Size: Toothed / 400 mm

Latitude: -43.610613 Longitude: 172.419367

					Bucket Type/Size :	oothed	/ 400	mm			Longitude	• : 1/2.4193	67
Depth (m BGL)	Material	Excavatability (Relative Scale) and education and educatio	USCS Symbol	DESCI	RIPTION	Graphic Symbol	Elevation (mRL)	Water Level	Moisture Cond.	Consistency/ Density Index	Shear Vane Peak/Remolded (kPa)		netrometer er 100mm 8 10 12
-	TOPSOIL		ML	Sandy SILT with trac Low plasticity. Sand [TOPSOIL].	fine to medium	1/ 3/1/ 1/ 3/1/ 1/ 5/1/				N/A		•	
0.5 - - - - - 1.0-			ML	SILT with some fine trace rootlets; light b	to medium sand and prown. Low plasticity.				D	VSt - H			Ä
1.5 -	ALLUVIUM		GW	cobbles and rootlets graded, rounded to to coarse, well grade	e GRAVEL with minor ; greyish brown. Well sub-angular. Sand fine ed. nger present at 1.2 m				M	Tightly Packed			
TA TEMPLATE.GDT 7/12				Depth of Excavation Termination Condition	: 2 m on: Target depth						-		
GEOTECH TEST PIT LOG ROLLESTON_TP_10_TO_18.GPJ NZ MASTER DATA TEMPLATE.GDT 7/12/20 S S S S S S S S S S S S S S S S S S S													-
Tes Sca Sta	ıla Po	reached target d enetrometer met g groundwater w	pract	cal refusal at 0.6 m c	lepth.								



APPENDIX 3:

Ecan Boreholes



Bore or Well No	BX23/0262
Well Name	Selwyn Road
Owner	MR & MRS J E & A T HEYL



Well Number	BX23/0262	File Number	CRC141970
Owner	MR & MRS J E & A T HEYL	Well Status	Active (exist, present)
Street/Road	Selwyn Road	NZTM Grid Reference	BX23:52560-70858
Locality	ROLLESTON	NZTM X and Y	1552560 - 5170858
Location Description		Location Accuracy	10 - 50m
CWMS Zone	Selwyn - Waihora	Use	Domestic Supply, Stock Supply
Groundwater Allocation Zone	Selwyn-Waimakariri	Water Level Monitoring	
Depth	42.00m	Water Level Count	1
Diameter	150mm	Initial Water Level	7.80m below MP
Measuring Point Description	Top of Casing	Highest Water Level	7.80m below MP
Measuring Point Elevation		Lowest Water Level	7.80m below MP
Elevation Accuracy		First reading	05 Nov 2013
Ground Level	0.50m below MP	Last reading	05 Nov 2013
Strata Layers	9	Calc Min 80%	
Aquifer Name		Aquifer Tests	0
Aquifer Type		Yield Drawdown Tests	1
Drill Date	05 Nov 2013	Max Tested Yield	
Driller	McMillan Drilling Ltd	Drawdown at Max Tested Yield	
Drilling Method	Rotary/Percussion	Specific Capacity	0.28 l/s/m
Casing Material	Steel	Last Updated	29 Nov 2013
Pump Type		Last Field Check	05 Nov 2013
Water Use Data	No		

Screens

Screen No.	Screen Type	Top (m)	Bottom (m)	Slot Size (mm)	Slot Length (mm)	Diameter (mm)	Leader Length (mm)
1	Stainless steel	40.48	42	2.5		125	

Step Tests

Step Test Date	Step	Yield	Yield GPM	DrawDown	Step Duration
05 Nov 2013	1	1.67	22.040966	5.93	1.25

Comments

Comment Date	Comment	
14 Nov 2013	NZMG Map Reference updated from: M36:62523-32589 shifted 124m	
14 Nov 2013	Driller confirms	

Bore Log

Borelog for well BX23/0262

Grid Reference (NZTM): 1552561 mE, 5170859 mN

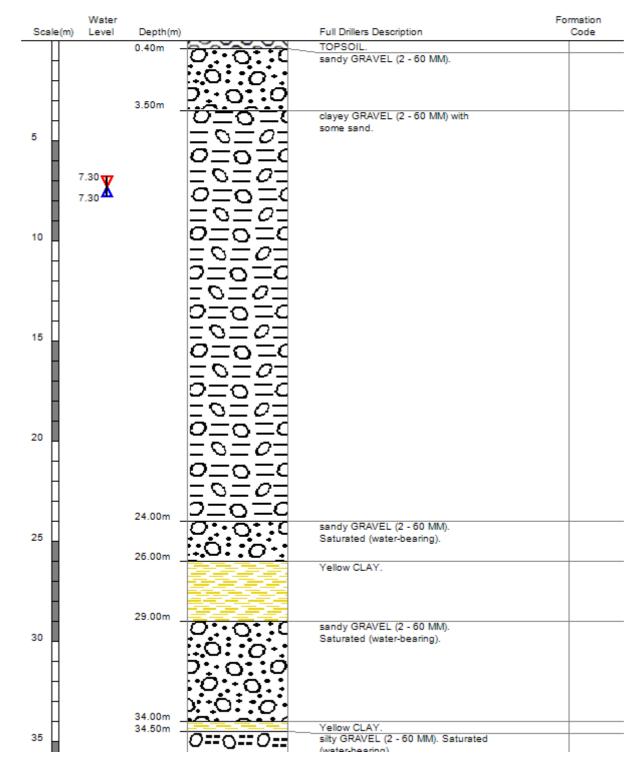
Location Accuracy: 10 - 50m

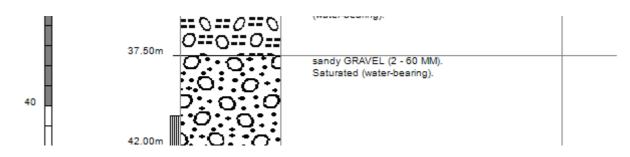
Ground Level Altitude: m +MSD Accuracy:

Driller: McMillan Drilling Ltd Drill Method: Rotary/Percussion

Borelog Depth: 42.0 m Drill Date: 05-Nov-2013



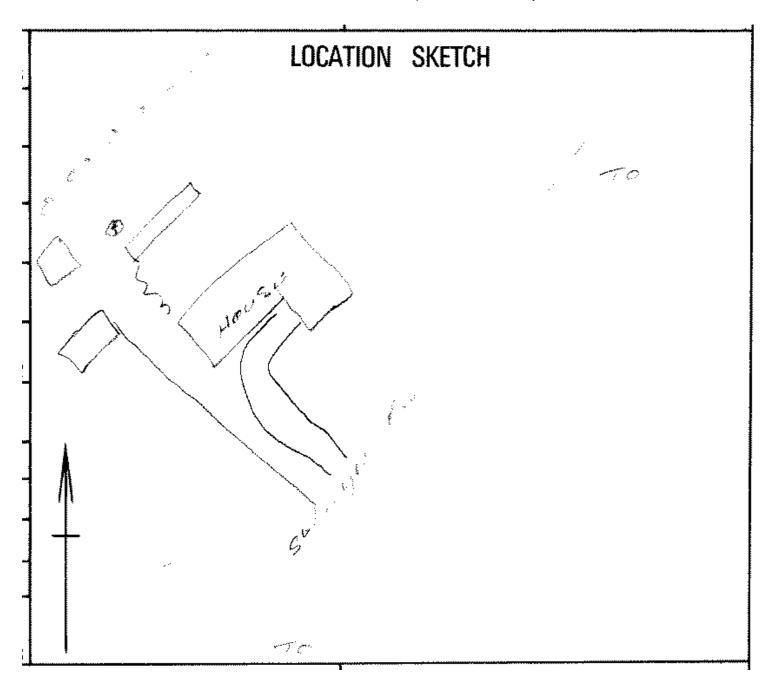




Bore or Well No	M36/1816
Well Name	SELWYN RD
Owner	MCLAUGHLAN



Well Number	M36/1816	File Number	
Owner	MCLAUGHLAN	Well Status	Not Used
Street/Road	SELWYN RD	NZTM Grid Reference	BX23:52806-70690
Locality	ROLLESTON	NZTM X and Y	1552806 - 5170690
Location Description		Location Accuracy	< 50m
CWMS Zone	Selwyn - Waihora	Use	,
Groundwater Allocation Zone	Selwyn-Waimakariri	Water Level Monitoring	
Depth	13.67m	Water Level Count	0
Diameter	150mm	Initial Water Level	7.64m below MP
Measuring Point Description		Highest Water Level	
Measuring Point Elevation	35.80m above MSL (Lyttelton 1937)	Lowest Water Level	
Elevation Accuracy	< 2.5 m	First reading	
Ground Level	0.00m above MP	Last reading	
Strata Layers	0	Calc Min 80%	9.27m below MP (Estimated)
Aquifer Name		Aquifer Tests	0
Aquifer Type	Unknown	Yield Drawdown Tests	0
Drill Date		Max Tested Yield	
Driller	McMillan Drilling Ltd	Drawdown at Max Tested Yield	
Drilling Method	Cable Tool	Specific Capacity	
Casing Material	STEEL	Last Updated	29 Jan 2007
Pump Type	Unknown	Last Field Check	
Water Use Data	No		



No screen data for this well

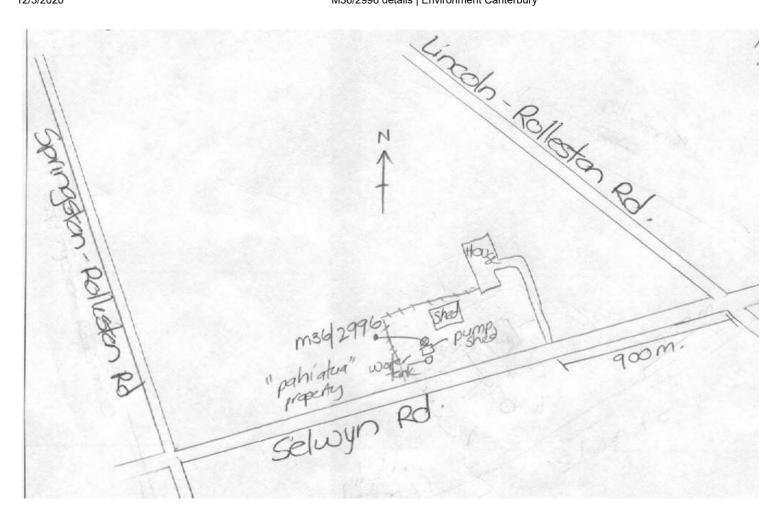
No step tests for this well

No comments for this well

Bore or Well No	M36/2996
Well Name	SELWYN ROAD
Owner	NISBET, NA & EM



Well Number	M36/2996	File Number	CO6C/01455
Owner	NISBET, NA & EM	Well Status	Active (exist, present)
Street/Road	SELWYN ROAD	NZTM Grid Reference	BX23:52913-70536
Locality	ROLLESTON	NZTM X and Y	1552913 - 5170536
Location Description	NEAR HAYSHED	Location Accuracy	2 - 15m
CWMS Zone	Selwyn - Waihora	Use	Irrigation, Domestic Supply
Groundwater Allocation Zone	Selwyn-Waimakariri	Water Level Monitoring	
Depth	59.40m	Water Level Count	0
Diameter	200mm	Initial Water Level	9.60m below MP
Measuring Point Description		Highest Water Level	
Measuring Point Elevation	35.67m above MSL (Lyttelton 1937)	Lowest Water Level	
Elevation Accuracy	< 2.5 m	First reading	
Ground Level	0.00m above MP	Last reading	
Strata Layers	0	Calc Min 80%	11.50m below MP (Estimated)
Aquifer Name	Linwood Gravel	Aquifer Tests	0
Aquifer Type	Unknown	Yield Drawdown Tests	1
Drill Date	15 Oct 1984	Max Tested Yield	11 l/s
Driller	McMillan Drilling Ltd	Drawdown at Max Tested Yield	14 m
Drilling Method	Rotary Rig	Specific Capacity	0.84 l/s/m
Casing Material	STEEL	Last Updated	08 Nov 2013
Pump Type	Unknown	Last Field Check	
Water Use Data	No		



Screens

Scree	n No.	Screen Type	Top (m)	Bottom (m)	Slot Size (mm)	Slot Length (mm)	Diameter (mm)	Leader Length (mm)
1		Stainless steel	55.7	59.4				

Step Tests

Step Test Date	Step	Yield	Yield GPM	DrawDown	Step Duration
15 Oct 1984	1	11.4	150.45929	13.6	18

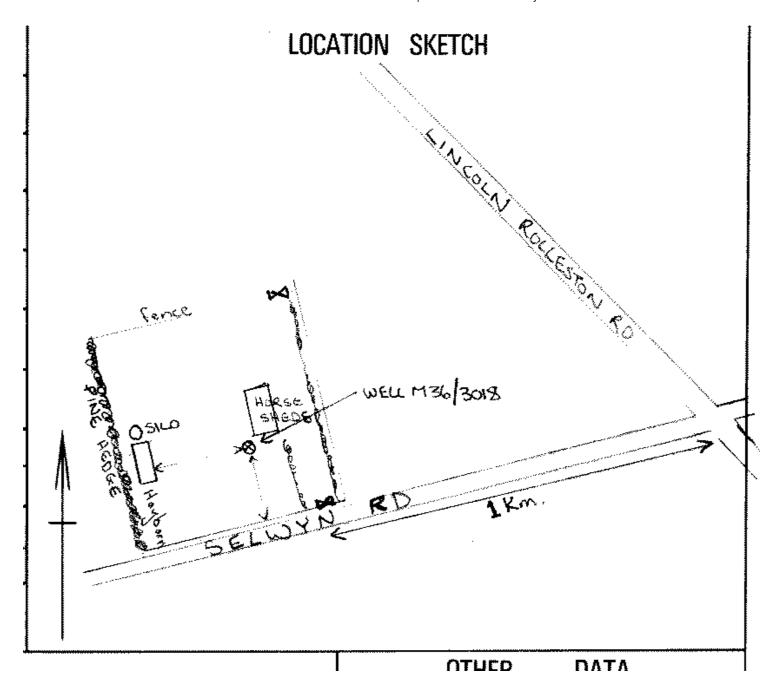
Comments

Comment Date	Comment
	NO LOG EXISTS. SCREENED IN MEDIUM STAINED GRAVELS.

Bore or Well No	M36/3018
Well Name	SELWYN RD
Owner	BOWDEN M.L.



Well Number	M36/3018	File Number	
Owner	BOWDEN M.L.	Well Status	Active (exist, present)
Street/Road	SELWYN RD	NZTM Grid Reference	BX23:52606-70440
Locality	ROLLESTON	NZTM X and Y	1552606 - 5170440
Location Description	NEAR STABLES	Location Accuracy	50 - 300m
CWMS Zone	Selwyn - Waihora	Use	Irrigation, Domestic and Stockwater
Groundwater Allocation Zone	Selwyn-Waimakariri	Water Level Monitoring	
Depth	65.70m	Water Level Count	0
Diameter	200mm	Initial Water Level	9.32m below MP
Measuring Point Description		Highest Water Level	
Measuring Point Elevation	34.95m above MSL (Lyttelton 1937)	Lowest Water Level	
Elevation Accuracy	< 2.5 m	First reading	
Ground Level	0.00m above MP	Last reading	
Strata Layers	9	Calc Min 80%	11.68m below MP (Estimated)
Aquifer Name	Linwood Gravel	Aquifer Tests	0
Aquifer Type	Unknown	Yield Drawdown Tests	1
Drill Date	13 Jan 1986	Max Tested Yield	16 l/s
Driller	McMillan Drilling Ltd	Drawdown at Max Tested Yield	22 m
Drilling Method	Rotary/Percussion	Specific Capacity	0.75 l/s/m
Casing Material	STEEL	Last Updated	08 Nov 2013
Pump Type	Submersible	Last Field Check	
Water Use Data	No		



Screens

Screen No.	Screen Type	Top (m)	Bottom (m)	Slot Size (mm)	Slot Length (mm)	Diameter (mm)	Leader Length (mm)
1	Stainless steel	58.4	61.3				
2	Stainless steel	63.3	65.4				

Step Tests

Step Test Date	Step	Yield	Yield GPM	DrawDown	Step Duration
13 Jan 1986	1	16.3	215.130386	21.7	14

Comments

Comment Date	Comment
	DRILLED TO 42M IN 1985.11.4l/s 23M DD

Bore Log

Borelog for well M36/3018

Grid Reference (NZTM): 1552607 mE, 5170441 mN

Location Accuracy: 50 - 300m

Ground Level Altitude: 35.0 m +MSD Accuracy: < 2.5 m

Driller: McMillan Drilling Ltd Drill Method: Rotary/Percussion

Borelog Depth: 65.7 m Drill Date: 13-Jan-1986



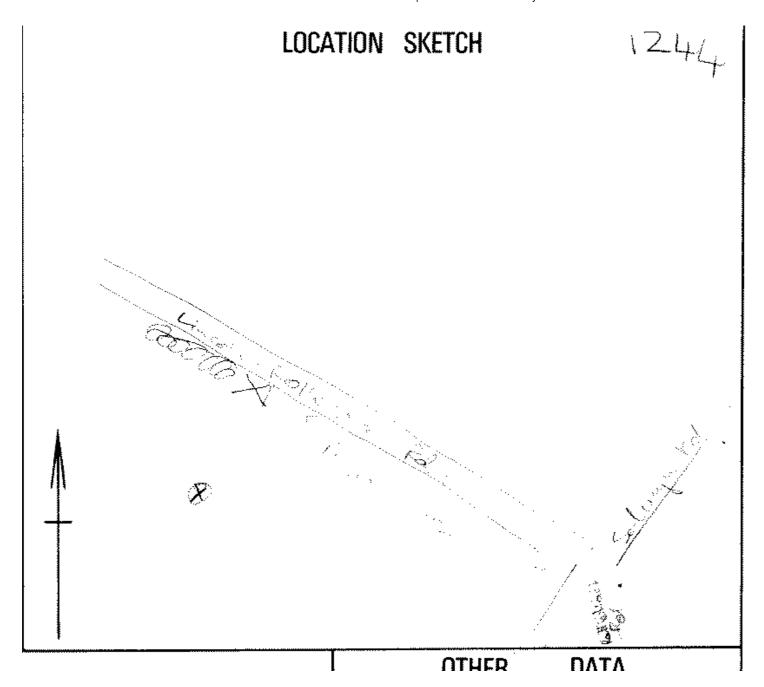
Scale(m)	Water Level	Depth(m)		Full Drillers Description	Formation Code
13 26		Depth(m)	No Log No Log No gg No Log No Log No No Log No Log No gg No Log No Log No gg No Log No Log No No Log No Log No No Log No Log No gg No Log No Log No gg No Log No Log No no Log No Log No gg No Log No Log No gg No Log No Log No gg No Log No Log No no Log No Log No gg No Log No Log No g No Log No Log No no Log No no Log No Log No no Log No no Log No no Log No no Log No	Full Drillers Description Not logged	
39		39.00m _	No Log No Log No No Log No Log No O:	Grey and Brown stained gravels, sandy	LI
		46.50m _	0:.0:.0:.0 :.0::0:.0	Grey and Brown Or stained gravels, very sandy and clayey	LI
53		53.00m _	000000	Tight claybound gravels	LI
		54.50m _	000	Grey and Brown Black stained gravels, fine sand and Orange/Yellow clays	LI

0:0::0::	Very hard claybound gravels, very fine sand	LI
<u>:::ō:::o:::ō</u>	Free Brown and Grey stained gravels, some clay and very sandy	LI
61.00m 00000	Hard claybound gravels	LI
0:.0:.0:.	Grey and Brown stained gravels, sandy and clayey	LI

Bore or Well No	M36/3836
Well Name	LINCOLN ROLLESTON RD
Owner	Mr & Mrs K M & J A Saulsbury



Well Number	M36/3836	File Number	CO6C/02068		
Owner	Mr & Mrs K M & J A Saulsbury	Well Status	Active (exist, present)		
Street/Road	LINCOLN ROLLESTON RD	NZTM Grid Reference	BX23:53106-71190		
Locality	BROADFIELD	NZTM X and Y	1553106 - 5171190		
Location Description		Location Accuracy	50 - 300m		
CWMS Zone	Selwyn - Waihora	Use	Irrigation,		
Groundwater Allocation Zone	Selwyn-Waimakariri	Water Level Monitoring			
Depth	56.60m	Water Level Count	0		
Diameter	220mm	Initial Water Level	10.70m below MP		
Measuring Point Description		Highest Water Level			
Measuring Point Elevation	37.24m above MSL (Lyttelton 1937)	Lowest Water Level			
Elevation Accuracy	< 2.5 m	First reading			
Ground Level	0.00m above MP	Last reading			
Strata Layers	8	Calc Min 80%	11.88m below MP (Estimated)		
Aquifer Name	Linwood Gravel	Aquifer Tests	0		
Aquifer Type	Unknown	Yield Drawdown Tests	1		
Drill Date	01 Nov 1987	Max Tested Yield	11 l/s		
Driller	McMillan Drilling Ltd	Drawdown at Max Tested Yield	14 m		
Drilling Method	Cable Tool	Specific Capacity	0.83 l/s/m		
Casing Material	STEEL	Last Updated	08 Nov 2013		
Pump Type	Submersible	Last Field Check			
Water Use Data	Yes				



Screens

Screen No.	Screen Type	Top (m)	Bottom (m)	Slot Size (mm)	Slot Length (mm)	Diameter (mm)	Leader Length (mm)
1	Stainless steel	53.6	56.6				

Step Tests

Step Test Date	Step	Yield	Yield GPM	DrawDown	Step Duration
01 Nov 1987	1	11.4	150.45929	13.7	4

Comments

Comment Date	Comment
21 Jul 2011	Previous owner VAN DE KLUNDERT A.W.

Bore Log

Borelog for well M36/3836

Grid Reference (NZTM): 1553107 mE, 5171191 mN

Location Accuracy: 50 - 300m

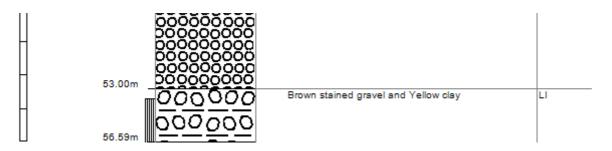
Ground Level Altitude: 37.2 m +MSD Accuracy: < 2.5 m

Driller: McMillan Drilling Ltd Drill Method: Cable Tool

Borelog Depth: 56.6 m Drill Date: 01-Nov-1987



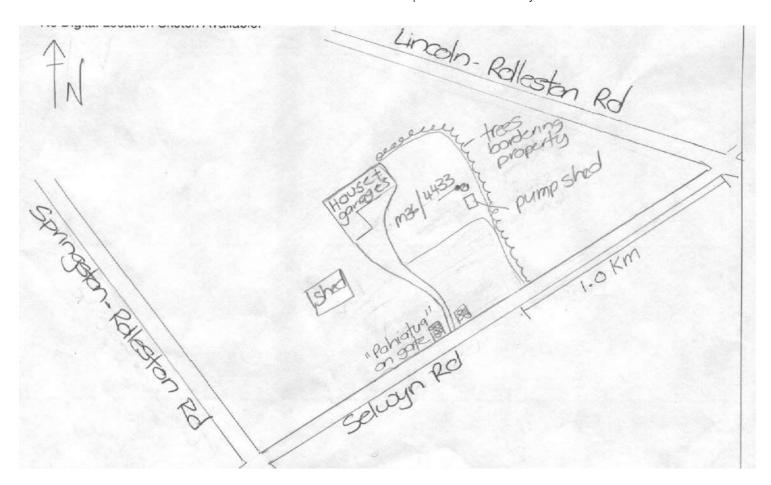
Scale(m)	Water Level	Depth(m)		Full Drillers Description	Formation Code
11		0.30m	10000	Earth	SP
				Brown clay	SP
11		2.50m	000000	~~	
34		33.50m	000000 000000 000000	Grey clay	BR?
		34.50m			
		38.40m	0:0::0::0	Light Brown gravel, very sandy with claywash	LI
		41.20m	000000	Small Brown gravel and some clay	LI
45		•	00000000	Medium sized Brown and Grey gravel	LI



Bore or Well No	M36/4433	
Well Name	SELWYN ROAD	
Owner	Mr & Mrs H J & D P Bates	



Well Number	M36/4433	File Number	CO6C/02646
Owner	Mr & Mrs H J & D P Bates	Well Status	Active (exist, present)
Street/Road	SELWYN ROAD	NZTM Grid Reference	BX23:52875-70540
Locality	ROLLESTON	NZTM X and Y	1552875 - 5170540
Location Description		Location Accuracy	2 - 15m
CWMS Zone	Selwyn - Waihora	Use	Irrigation, Domestic Supply
Groundwater Allocation Zone	Selwyn-Waimakariri	Water Level Monitoring	
Depth	30.00m	Water Level Count	0
Diameter	150mm	Initial Water Level	9.70m below MP
Measuring Point Description		Highest Water Level	
Measuring Point Elevation	35.61m above MSL (Lyttelton 1937)	Lowest Water Level	
Elevation Accuracy	< 2.5 m	First reading	
Ground Level	0.00m above MP	Last reading	
Strata Layers	10	Calc Min 80%	12.08m below MP (Estimated)
Aquifer Name	Bromley Formation	Aquifer Tests	0
Aquifer Type	Unknown	Yield Drawdown Tests	1
Drill Date	20 Dec 1991	Max Tested Yield	2 l/s
Driller	Smiths Welldrilling	Drawdown at Max Tested Yield	4 m
Drilling Method	Cable Tool	Specific Capacity	0.52 l/s/m
Casing Material		Last Updated	08 Nov 2013
Pump Type	Unknown	Last Field Check	
Water Use Data	No		



Screens

Screen No.	Screen Type	Top (m)	Bottom (m)	Slot Size (mm)	Slot Length (mm)	Diameter (mm)	Leader Length (mm)
1	Stainless steel	28	30				

Step Tests

Step Test Date	Step	Yield	Yield GPM	DrawDown	Step Duration
20 Dec 1991	1	2.2	29.036005	4.2	2

Comments

Comment Date	Comment
	Previous owner S Matheson.

Borelog for well M36/4433

Grid Reference (NZTM): 1552876 mE, 5170541 mN

Location Accuracy: 2 - 15m

Ground Level Altitude: 35.6 m +MSD Accuracy: < 2.5 m

Driller: Smiths Welldrilling Drill Method: Cable Tool

Borelog Depth: 30.0 m Drill Date: 20-Dec-1991



	Water				Formation
Scale(m)	Level	Depth(m)		Full Drillers Description	Code
- 11		0.25m -		Earth	SP? SP?
5		0.60m 7		Clay Large rough stones and sand	SP-RI
10			000000 00000 00000 00000 00000 00000	Claybound small to medium gravel	RI
20		17.50m _	0.0.0.0	Claybound gravel and sand	RI
		23.00m	000000000000000000000000000000000000000	Some water	RI
25		_	000000 000000	Claybound	RI



Bore or Well No	M36/7975
Well Name	SELWYN ROAD
Owner	MR & MS SH & EL LOEFFLER & HUISMANS



Well Number	M36/7975	File Number	CO6C/23830
Owner	MR & MS SH & EL LOEFFLER & HUISMANS	Well Status	Active (exist, present)
Street/Road	SELWYN ROAD	NZTM Grid Reference	BX23:52316-71000
Locality	ROLLESTON	NZTM X and Y	1552316 - 5171000
Location Description		Location Accuracy	50 - 300m
CWMS Zone	Selwyn - Waihora	Use	Domestic Supply,
Groundwater Allocation Zone	Selwyn-Waimakariri	Water Level Monitoring	
Depth	37.50m	Water Level Count	0
Diameter	150mm	Initial Water Level	10.00m below MP
Measuring Point Description	ТоС	Highest Water Level	
Measuring Point Elevation	38.00m above MSL (Lyttelton 1937)	Lowest Water Level	
Elevation Accuracy	< 2.5 m	First reading	
Ground Level	0.30m below MP	Last reading	
Strata Layers	12	Calc Min 80%	12.03m below MP (Estimated)
Aquifer Name		Aquifer Tests	0
Aquifer Type		Yield Drawdown Tests	3
Drill Date	05 Sep 2005	Max Tested Yield	6 l/s
Driller	Dynes Road Drilling	Drawdown at Max Tested Yield	17 m
Drilling Method	Cable Tool	Specific Capacity	0.43 l/s/m
Casing Material	Steel	Last Updated	08 Nov 2013
Pump Type		Last Field Check	
Water Use Data	No		

Screen No.	Screen Type	Top (m)	Bottom (m)	Slot Size (mm)	Slot Length (mm)	Diameter (mm)	Leader Length (mm)
1	Stainless steel	35	37.5				

Step Test Date	Step	Yield	Yield GPM	DrawDown	Step Duration
05 Sep 2005	1	3.636864	48	8.5	1
05 Sep 2005	2	5.379528	71	12.2	1

Step Test Date	Step	Yield	Yield GPM	DrawDown	Step Duration
05 Sep 2005	3	6.364512	84	17.3	2

No comments for this well

Borelog for well M36/7975

Grid Reference (NZTM): 1552317 mE, 5171001 mN

Location Accuracy: 50 - 300m

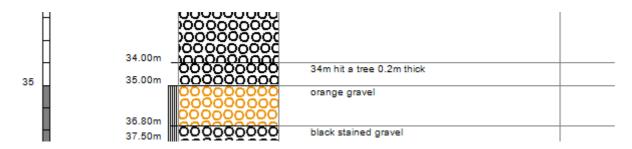
Ground Level Altitude: 37.7 m +MSD Accuracy: < 2.5 m

Driller: Dynes Road Drilling Drill Method: Cable Tool

Borelog Depth: 37.5 m Drill Date: 05-Sep-2005



	Water				Formation
Scale(m)	Level	Depth(m)		Full Drillers Description	Code
		0.70m		brown topsoil	
Н			0==0==0==	small-med gravel some silt	
Ц			F= 0 == 0 == 0		
- 11			D=0		
Н			000		
Ц			<u> </u>		
_			0==0==		
5			=0=0=0		
Щ			0==0==0==		
- 1					
н					
Щ			00		
- 1			E0=0=0		
H			D==0==0==		
10			-0==0==0		
			heine nei		
Н			000		
Н		12.00m _	0000000000000	cool sounded cools	
- 11			000000000000000000000000000000000000000	small rounded gravel	
П			000000000000000000000000000000000000000		
Н			0000000000000		
15			0000000000000		
			100000000000		
H			100000000000000000000000000000000000000		
- 1			000000000000		
			200000000000000000000000000000000000000		
H			10000000000C		
		19.00m	200000000000		
		_	000000000	small-med subrounded gravel	
20			000000000		
			000000000		
П			000000000		
Н			0000000000		
- 11			1000000000		
П			000000000		
Н			000000000		
25		25.00m	000000000		
		_		solid yellow silt water sealing	
н		26.00m _	00000000	small-med rounded gravel - some	
			000000000	stained	
			000000000		
Н		28.00m _ 28.50m _	000000000	some sand with gravel	
		20.0UM _	00000000	small rounded stained gravel	
		20.05	000000000		
30		30.00m	000000000	loose rounded med gravel	
			000000000	loose rounded med graver	
П			pogogodod		
					1



Bore or Well No	M36/7976
Well Name	SELWYN ROAD
Owner	WEATHERBY ESTATE LIMITED



	M00/7070	·	0000/0007
Well Number	M36/7976	File Number	CO6C/23897
Owner	WEATHERBY ESTATE LIMITED	Well Status	Active (exist, present)
Street/Road	SELWYN ROAD	NZTM Grid Reference	BX23:52386-70950
Locality	BROADFIELD	NZTM X and Y	1552386 - 5170950
Location Description		Location Accuracy	10 - 50m
CWMS Zone	Selwyn - Waihora	Use	Domestic and Stockwater,
Groundwater Allocation Zone	Selwyn-Waimakariri	Water Level Monitoring	
Depth	36.00m	Water Level Count	0
Diameter	150mm	Initial Water Level	10.60m below MP
Measuring Point Description	ToC	Highest Water Level	
Measuring Point Elevation	37.00m above MSL (Lyttelton 1937)	Lowest Water Level	
Elevation Accuracy	< 2.5 m	First reading	
Ground Level	0.40m below MP	Last reading	
Strata Layers	11	Calc Min 80%	12.04m below MP (Estimated)
Aquifer Name		Aquifer Tests	0
Aquifer Type		Yield Drawdown Tests	1
Drill Date	24 Aug 2005	Max Tested Yield	2 l/s
Driller	Daly Water Wells Ltd	Drawdown at Max Tested Yield	3 m
Drilling Method	Rotary Rig	Specific Capacity	0.63 l/s/m
Casing Material	Steel	Last Updated	08 Nov 2013
Pump Type		Last Field Check	
Water Use Data	No		

Screen No.	Screen Type	Top (m)	Bottom (m)	Slot Size (mm)	Slot Length (mm)	Diameter (mm)	Leader Length (mm)
1	Stainless steel	34	36				

Step Test Date	Step	Yield	Yield GPM	DrawDown	Step Duration
24 Aug 2005	1	2	26.396368	3.2	4

Comment Date	Comment
28 Mar 2006	Gridref changed from: M36:6233-3268

Borelog for well M36/7976

Grid Reference (NZTM): 1552387 mE, 5170951 mN

Location Accuracy: 10 - 50m

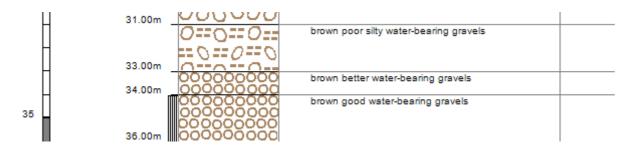
Ground Level Altitude: 36.6 m +MSD Accuracy: < 2.5 m

Driller: Daly Water Wells Ltd Drill Method: Rotary Rig

Borelog Depth: 36.0 m Drill Date: 24-Aug-2005



Scale(m)	Water Level	Depth(m)		Full Drillers Description	Formation Code
		0.30m -	66868	brown topsoil	
Н			000000000	grey dry clean gravels	
Н		0.50	000000000		
Ц		2.50m _ 3.00m _	70:00:00	grey sandy moist gravels	
			000000	brown claybound gravels	
5			000000		
			000000		
			000000		
H			000000		
H					
H			000000		
10			000000		
Н			000000		
Ц		12.00m _	200000		
Ц			0==0==0==	brown silty claybound gravels	
			==0==0		
15		15.00m	0==0==0==		
. [_	000000	grey heavy claybound gravels	
			000000		
Ħ			000000		
H			000000		
H					
20			000000		
Н		21.00m _	Dogue	grey sandy gravels traces of clay	
Н				grey samely graves makes or day	
Ц					
Ц			D:-0::0::		
25					
			P: 20 : 19:21		
		20.00	0::0::0		
		28.00m _	000000	brown claybound gravels	
			200000		
30					



Bore or Well No	M36/8002
Well Name	SELWYN ROAD
Owner	MR & MRS AT & JM MULCAY



Well Number	M36/8002	File Number	CO6C/24021
Owner	MR & MRS AT & JM MULCAY	Well Status	Active (exist, present)
Street/Road	SELWYN ROAD	NZTM Grid Reference	BX23:52536-70700
Locality	ROLLESTON	NZTM X and Y	1552536 - 5170700
Location Description		Location Accuracy	50 - 300m
CWMS Zone	Selwyn - Waihora	Use	Domestic and Stockwater,
Groundwater Allocation Zone	Selwyn-Waimakariri	Water Level Monitoring	
Depth	66.00m	Water Level Count	0
Diameter	150mm	Initial Water Level	12.40m below MP
Measuring Point Description	ToC	Highest Water Level	
Measuring Point Elevation	36.00m above MSL (Lyttelton 1937)	Lowest Water Level	
Elevation Accuracy	< 2.5 m	First reading	
Ground Level	0.30m below MP	Last reading	
Strata Layers	13	Calc Min 80%	11.96m below MP (Estimated)
Aquifer Name		Aquifer Tests	0
Aquifer Type		Yield Drawdown Tests	1
Drill Date	28 Oct 2005	Max Tested Yield	4 l/s
Driller	Smiths Welldrilling	Drawdown at Max Tested Yield	11 m
Drilling Method	Rotary/Percussion	Specific Capacity	0.37 l/s/m
Casing Material	Steel	Last Updated	08 Nov 2013
Pump Type		Last Field Check	
Water Use Data	No		

Screen No.	Screen Type	Top (m)	Bottom (m)	Slot Size (mm)	Slot Length (mm)	Diameter (mm)	Leader Length (mm)
1	Stainless steel	64.5	66				

Step Test Date	Step	Yield	Yield GPM	DrawDown	Step Duration
30 Oct 2005	1	4.06	53.5846252	11	2

Comment Date	Comment
04 May 2007	Gridref changed from: M36:6249-3232, original gridref incorrect (on road) still waiting for BCR

Borelog for well M36/8002

Grid Reference (NZTM): 1552537 mE, 5170701 mN

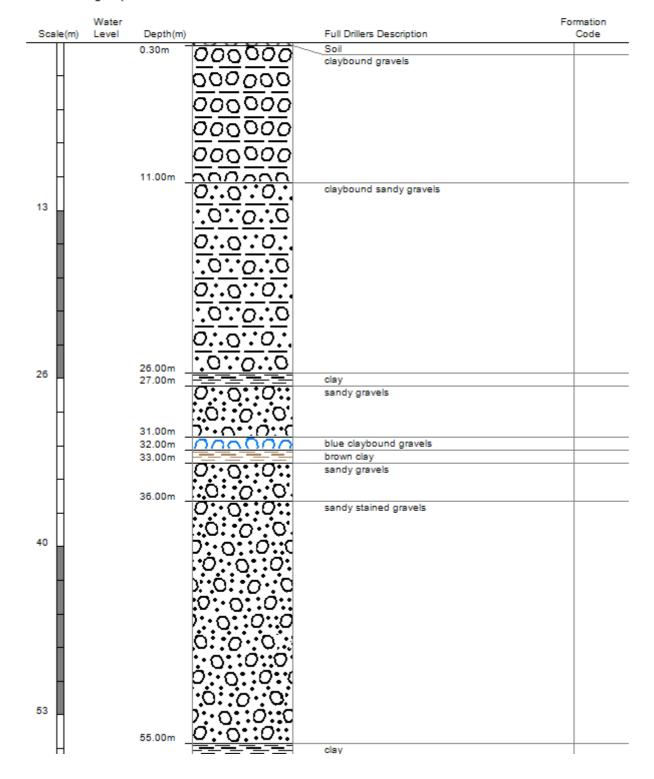
Location Accuracy: 50 - 300m

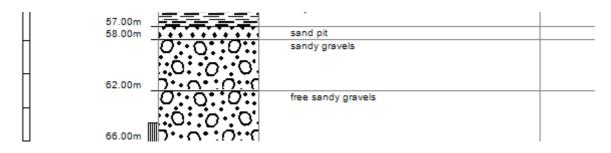
Ground Level Altitude: 35.7 m +MSD Accuracy: < 2.5 m

Driller: Smiths Welldrilling Drill Method: Rotary/Percussion

Borelog Depth: 66.0 m Drill Date: 28-Oct-2005







Bore or Well No	M36/8009
Well Name	SELWYN ROAD
Owner	MR & MRS GC & PM MORGAN



Well Number	M36/8009	File Number	CO6C/24231
Owner	MR & MRS GC & PM MORGAN	Well Status	Active (exist, present)
Street/Road	SELWYN ROAD	NZTM Grid Reference	BX23:52356-70860
Locality	BROADFIELD	NZTM X and Y	1552356 - 5170860
Location Description		Location Accuracy	50 - 300m
CWMS Zone	Selwyn - Waihora	Use	Domestic and Stockwater,
Groundwater Allocation Zone	Selwyn-Waimakariri	Water Level Monitoring	
Depth	36.00m	Water Level Count	0
Diameter	150mm	Initial Water Level	12.20m below MP
Measuring Point Description	ТоС	Highest Water Level	
Measuring Point Elevation	36.18m above MSL (Lyttelton 1937)	Lowest Water Level	
Elevation Accuracy	< 5 m	First reading	
Ground Level	0.40m below MP	Last reading	
Strata Layers	10	Calc Min 80%	11.92m below MP (Estimated)
Aquifer Name		Aquifer Tests	0
Aquifer Type		Yield Drawdown Tests	1
Drill Date	01 Nov 2005	Max Tested Yield	2 l/s
Driller	Daly Water Wells Ltd	Drawdown at Max Tested Yield	3 m
Drilling Method	Rotary Rig	Specific Capacity	0.63 l/s/m
Casing Material	Steel	Last Updated	08 Nov 2013
Pump Type		Last Field Check	
Water Use Data	No		

Screen No.	Screen Type	Top (m)	Bottom (m)	Slot Size (mm)	Slot Length (mm)	Diameter (mm)	Leader Length (mm)
1	Slotted PVC	34	36				

Step Test Date	Step	Yield	Yield GPM	DrawDown	Step Duration
01 Nov 2006	1	2	26.396368	3.2	3

No comments for this well

Borelog for well M36/8009 page 1 of 2

Grid Reference (NZTM): 1552357 mE, 5170861 mN

Location Accuracy: 50 - 300m

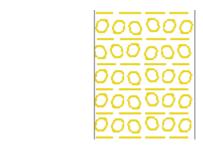
Ground Level Altitude: 35.8 m +MSD Accuracy: < 0.5 m

Driller: Daly Water Wells Ltd Drill Method: Rotary Rig

Borelog Depth: 36.0 m Drill Date: 01-Nov-2005



Water Scale(m) Level	Depth(m)		Full Drillers Description	Formation Code
	0.25m _	676767676	brown top soil	
	0.25m		brown top soil	
	0.60m -	000000000	yellow/brown dry clay	
Н	0.60m	000000000	yellow/brown dry clay grey dry gravels small-large (swl =	
		000000000	11.8m)	
		1000000000	,	
Н		0000000000		
		100000000000		
		000000000		
Н		000000000		
		000000000		
		0000000000		
Ц		000000000		
		5000000000		
	4.70m	000000000		
5	4.70m	000000	grey dry gravels small-large (swl = 11.8m)	
		000000	yellow small-medium claybound gravels	
H		000000		
		000000		
H		000000		
		000000		
H		000000		
		000000		
- 1		000000		
10		000000		
[000000		
Ц		000000		
		000000		
		000000		
		000000		
Ц		000000		
		000000		
		200		
		50000		
		000000		
15		000000 000000 000000		



Bore or Well No	M36/8138
Well Name	LINCOLN ROLLESTON ROAD
Owner	MR & MRS RG & VA HUBBARD



Well Number	M36/8138	File Number	CO6C/24642
Owner	MR & MRS RG & VA HUBBARD	Well Status	Active (exist, present)
Street/Road	LINCOLN ROLLESTON ROAD	NZTM Grid Reference	BX23:52946-71200
Locality	ROLLESTON	NZTM X and Y	1552946 - 5171200
Location Description		Location Accuracy	10 - 50m
CWMS Zone	Selwyn - Waihora	Use	Domestic and Stockwater,
Groundwater Allocation Zone	Selwyn-Waimakariri	Water Level Monitoring	
Depth	36.00m	Water Level Count	0
Diameter	150mm	Initial Water Level	14.60m below MP
Measuring Point Description	ToC	Highest Water Level	
Measuring Point Elevation	35.69m above MSL (Lyttelton 1937)	Lowest Water Level	
Elevation Accuracy	< 5 m	First reading	
Ground Level	0.40m below MP	Last reading	
Strata Layers	5	Calc Min 80%	12.83m below MP (Estimated)
Aquifer Name		Aquifer Tests	0
Aquifer Type		Yield Drawdown Tests	1
Drill Date	16 Jan 2006	Max Tested Yield	2 l/s
Driller	Daly Water Wells Ltd	Drawdown at Max Tested Yield	4 m
Drilling Method	Rotary Rig	Specific Capacity	0.51 l/s/m
Casing Material	Steel	Last Updated	08 Nov 2013
Pump Type		Last Field Check	
Water Use Data	No		

Screen No.	Screen Type	Top (m)	Bottom (m)	Slot Size (mm)	Slot Length (mm)	Diameter (mm)	Leader Length (mm)
1	Stainless steel	34	36				

Step Test Date	Step	Yield	Yield GPM	DrawDown	Step Duration
16 Jan 2006	1	2	26.396368	3.9	0

Comment Date	Comment
27 May 2008	Gridref changed from: M36:6294-3282 (BCP received)

Borelog for well M36/8138

Grid Reference (NZTM): 1552947 mE, 5171201 mN

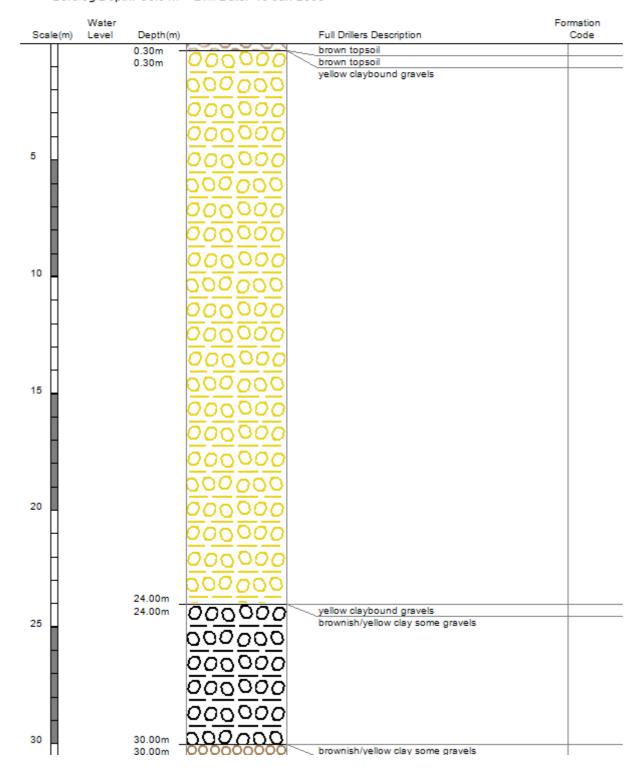
Location Accuracy: 10 - 50m

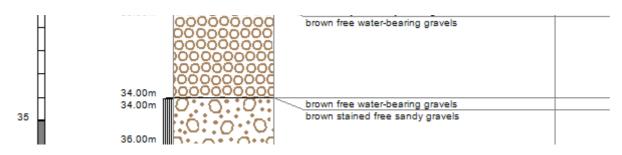
Ground Level Altitude: 35.3 m +MSD Accuracy: < 0.5 m

Driller: Daly Water Wells Ltd Drill Method: Rotary Rig

Borelog Depth: 36.0 m Drill Date: 16-Jan-2006







Bore or Well No	M36/8299
Well Name	SELWYN ROAD
Owner	MR GJ & MRS BA SCURR



Well Number	M36/8299	File Number	CO6C/24910
Owner	MR GJ & MRS BA SCURR	Well Status	Active (exist, present)
Street/Road	SELWYN ROAD	NZTM Grid Reference	BX23:52686-70750
Locality	BROADFIELDS	NZTM X and Y	1552686 - 5170750
Location Description		Location Accuracy	10 - 50m
CWMS Zone	Selwyn - Waihora	Use	Domestic Supply,
Groundwater Allocation Zone	Selwyn-Waimakariri	Water Level Monitoring	
Depth	90.00m	Water Level Count	0
Diameter	150mm	Initial Water Level	21.00m below MP
Measuring Point Description	ToC	Highest Water Level	
Measuring Point Elevation	34.56m above MSL (Lyttelton 1937)	Lowest Water Level	
Elevation Accuracy	< 5 m	First reading	
Ground Level	0.60m below MP	Last reading	
Strata Layers	7	Calc Min 80%	11.87m below MP (Estimated)
Aquifer Name		Aquifer Tests	0
Aquifer Type		Yield Drawdown Tests	0
Drill Date	20 Feb 2008	Max Tested Yield	
Driller	Giltrap Drilling	Drawdown at Max Tested Yield	
Drilling Method	Rotary Rig	Specific Capacity	
Casing Material	Steel	Last Updated	19 Mar 2010
Pump Type		Last Field Check	
Water Use Data	No		

Screen No.	Screen Type	Top (m)	Bottom (m)	Slot Size (mm)	Slot Length (mm)	Diameter (mm)	Leader Length (mm)
1	Stainless steel	88.5	90				

No step tests for this well

Comment Date	Comment
19 Mar 2010	Gridref changed from: M36:6266-3237 to M36:6268-3236 BCP confirms

Borelog for well M36/8299

Grid Reference (NZTM): 1552687 mE, 5170751 mN

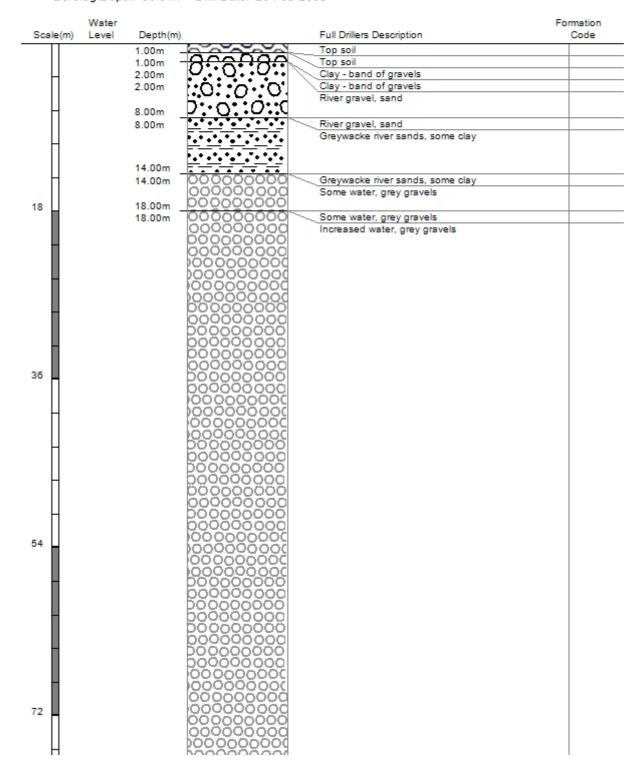
Location Accuracy: 10 - 50m

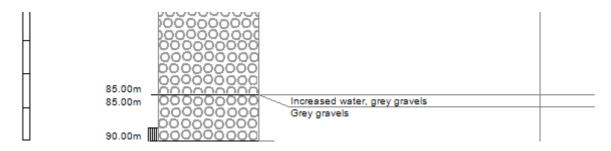
Ground Level Altitude: 34.0 m +MSD Accuracy: < 0.5 m

Driller: Giltrap Drilling Drill Method: Rotary Rig

Borelog Depth: 90.0 m Drill Date: 20-Feb-2008







Bore or Well No	M36/8300
Well Name	SELWYN ROAD
Owner	Mr & Mrs G J & B A Scurr



Well Number	M36/8300	File Number	CO6C/24910
Owner	Mr & Mrs G J & B A Scurr	Well Status	Active (exist, present)
Street/Road	SELWYN ROAD	NZTM Grid Reference	BX23:52676-70960
Locality	BROADFIELDS	NZTM X and Y	1552676 - 5170960
Location Description		Location Accuracy	10 - 50m
CWMS Zone	Selwyn - Waihora	Use	Domestic Supply,
Groundwater Allocation Zone	Selwyn-Waimakariri	Water Level Monitoring	
Depth	42.00m	Water Level Count	0
Diameter	150mm	Initial Water Level	14.00m below MP
Measuring Point Description	ToC	Highest Water Level	
Measuring Point Elevation	35.29m above MSL (Lyttelton 1937)	Lowest Water Level	
Elevation Accuracy	< 5 m	First reading	
Ground Level	0.60m below MP	Last reading	
Strata Layers	5	Calc Min 80%	12.33m below MP (Estimated)
Aquifer Name		Aquifer Tests	0
Aquifer Type		Yield Drawdown Tests	0
Drill Date	25 Feb 2008	Max Tested Yield	
Driller	Giltrap Drilling	Drawdown at Max Tested Yield	
Drilling Method	Rotary Rig	Specific Capacity	
Casing Material	Steel	Last Updated	19 May 2010
Pump Type		Last Field Check	
Water Use Data	Yes		

Screen No.	Screen Type	Top (m)	Bottom (m)	Slot Size (mm)	Slot Length (mm)	Diameter (mm)	Leader Length (mm)
1	Stainless steel	39	40.5				

No step tests for this well

Comment Date	Comment
19 Mar 2010	Gridref changed from: M36:6262-3256 to M36:6267-3257 BCP confirms

Borelog for well M36/8300

Grid Reference (NZTM): 1552677 mE, 5170961 mN

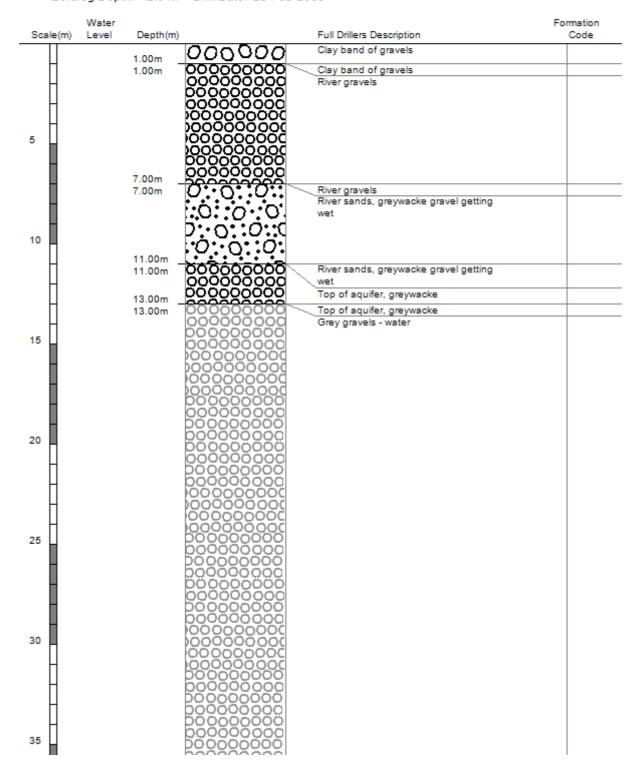
Location Accuracy: 10 - 50m

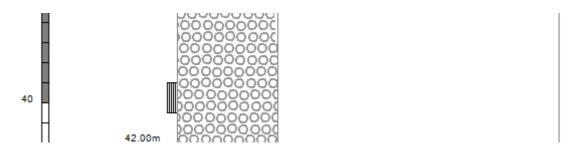
Ground Level Altitude: 34.7 m +MSD Accuracy: < 0.5 m

Driller: Giltrap Drilling Drill Method: Rotary Rig

Borelog Depth: 42.0 m Drill Date: 25-Feb-2008







Bore or Well No	M36/8392
Well Name	572 SELWYN ROAD
Owner	A J LLOYD



Well Number	M36/8392	File Number	CO6C/26653
Owner	A J LLOYD	Well Status	Active (exist, present)
Street/Road	572 SELWYN ROAD	NZTM Grid Reference	BX23:52567-70574
Locality	SPRINGSTON	NZTM X and Y	1552567 - 5170574
Location Description		Location Accuracy	2 - 15m
CWMS Zone	Selwyn - Waihora	Use	Domestic Supply,
Groundwater Allocation Zone	Selwyn-Waimakariri	Water Level Monitoring	
Depth	36.00m	Water Level Count	0
Diameter	150mm	Initial Water Level	10.00m below MP
Measuring Point Description		Highest Water Level	
Measuring Point Elevation	34.26m above MSL (Lyttelton 1937)	Lowest Water Level	
Elevation Accuracy	< 5 m	First reading	
Ground Level	0.00m above MP	Last reading	
Strata Layers	5	Calc Min 80%	11.83m below MP (Estimated)
Aquifer Name		Aquifer Tests	0
Aquifer Type		Yield Drawdown Tests	1
Drill Date	12 Jun 2007	Max Tested Yield	2 l/s
Driller	Daly Water Wells Ltd	Drawdown at Max Tested Yield	11 m
Drilling Method	Rotary Rig	Specific Capacity	0.18 l/s/m
Casing Material	Steel	Last Updated	08 Nov 2013
Pump Type		Last Field Check	
Water Use Data	No		

Screen No.	Screen Type	Top (m)	Bottom (m)	Slot Size (mm)	Slot Length (mm)	Diameter (mm)	Leader Length (mm)
1	Stainless steel	34	36				

Step Test Date	Step	Yield	Yield GPM	DrawDown	Step Duration
12 Jun 2007	1	2	26.396368	11	0

Comment Date	Comment
20 Sep 2007	Gridref changed from: M36:6247-3217, BCR confirms
12 Nov 2007	Gridref changed from: M36:6255-3216. New gridref from plan in BCR
09 Jun 2009	Gridref changed from: M36:62572-32149 - Site visit 5 June 09, location GPS'd

Borelog for well M36/8392

Grid Reference (NZTM): 1552568 mE, 5170575 mN

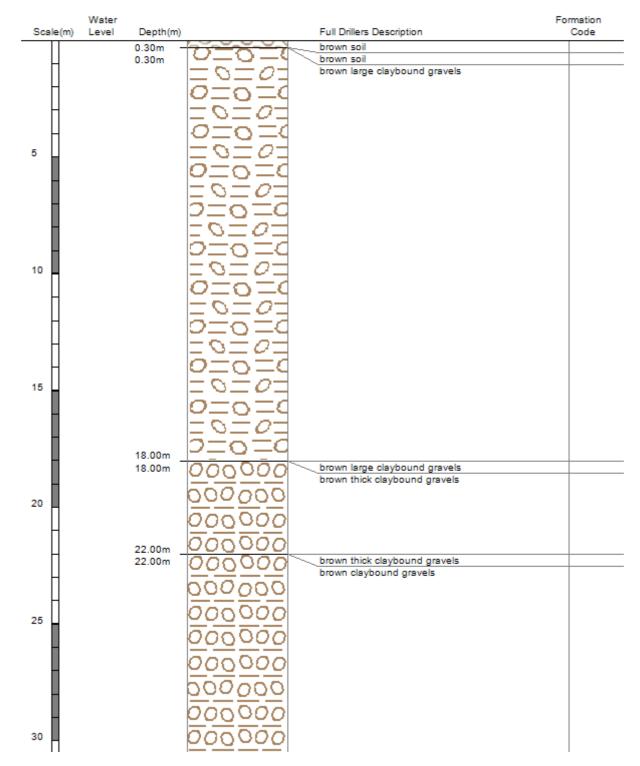
Location Accuracy: 2 - 15m

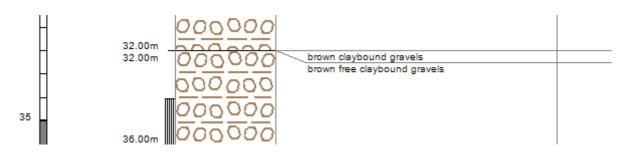
Ground Level Altitude: 34.3 m +MSD Accuracy: < 0.5 m

Driller: Daly Water Wells Ltd Drill Method: Rotary Rig

Borelog Depth: 36.0 m Drill Date: 12-Jun-2007









APPENDIX 3:

Site Plan and Subsurface Investigation







Geotechnical Investigation Broadfield Grange 572 Selwyn Road, Rolleston 18991.001.001

Client: Broadfield Grange LTD Date: 02/06/2022

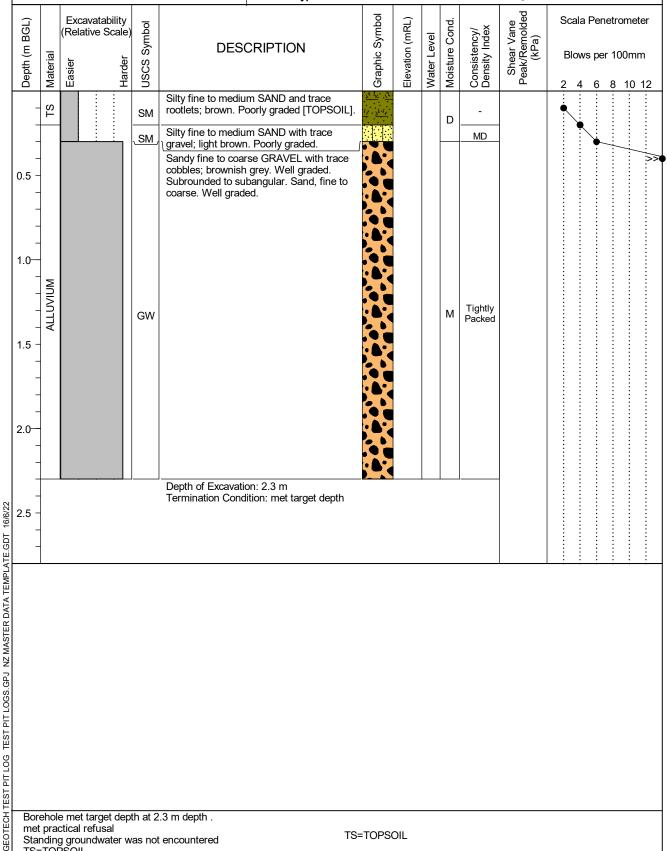
Shear Vane No: Logged By: Cw/JC

Max Test Pit Depth: 2.3 m

Reviewed By : JRW

Digger Type/Size : Bucket Excavator Bucket Type/Size: 400 mm

Latitude: -43.61701940326 Longitude: 172.41185556437





Geotechnical Investigation Broadfield Grange 572 Selwyn Road, Rolleston 18991.001.001

Client: Broadfield Grange LTD Date: 02/06/2022

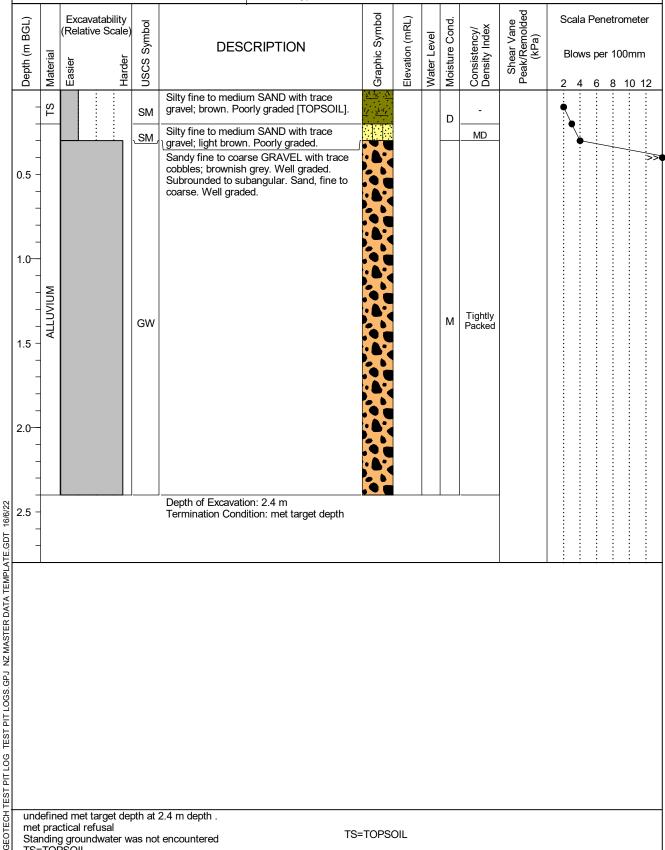
Shear Vane No: Logged By : JC/CW

Max Test Pit Depth: 2.4 m

Reviewed By : JRW

Digger Type/Size : Bucket Excavator Bucket Type/Size: 400 mm

Latitude: -43.616459647077 Longitude: 172.41047406583



undefined met target depth at 2.4 m depth . met practical refusal Standing groundwater was not encountered TS=TOPSOIL

TS=TOPSOIL



Geotechnical Investigation Broadfield Grange 572 Selwyn Road, Rolleston 18991.001.001

Client: Broadfield Grange LTD

Shear Vane No :

Date : 02/06/2022 Max Test Pit Depth : 2.6 m

 $\textbf{Logged By}: \mathsf{CW/JC}$ Reviewed By : JRW

Digger Type/Size : Bucket Excavator Bucket Type/Size : 400 mm

Latitude: -43.615517788612 **Longitude**: 172.4108900288

					Bucket Type/Size : 4	·uu mm					Longitude	e: 1/2.410	J890028	,δ
Depth (m BGL)	Material	Excavatability (Relative Scale) is is i	USCS Symbol	DESC	CRIPTION	Graphic Symbol	Elevation (mRL)	Water Level	Moisture Cond.	Consistency/ Density Index	Shear Vane Peak/Remolded (kPa)	Blows	enetromper 100r	mm
	TS		ML	Sandy SILT with to brown. Low plastic medium. Poorly gr	race gravel and rootlets; city. Sand, fine to raded ITOPSOIL1.	$\frac{1}{N_1} \cdot \frac{N_1 \cdot 1^{\lambda_1}}{N_1 \cdot N_1}$				-		•		
			SM		m SAND with trace					MD				
0.5	_ - - -			Sandy fine to coar cobbles; brownish Subrounded to su coarse. Well grade	rse GRAVEL with trace grey. Well graded. bangular. Sand, fine to ed.								•	^^
1.0-	- - - - - V													
1.5	ALLUVIUM		GW						M	Tightly Packed				
2.0	- - -					XX								
2.5						容								
EMPLATE.GDT 16/6/22	-]	Depth of Excavation Termination Cond	on: 2.6 m ition: met target depth									
GEOTECH TEST PIT LOG TEST PIT LOGS.GPJ NZ MASTER DATA TEMPLATI													: : :	÷
une EOTECH I me	et pra	ed met target de ctical refusal g groundwater w			TS	=TOPS	OIL							
- - 18	- 1∪ 1	'OUIL												



Geotechnical Investigation Broadfield Grange 572 Selwyn Road, Rolleston 18991.001.001 Client: Broadfield Grange LTD

Shear Vane No :

Date : 02/06/2022 **Max Test Pit Depth** : 2.3 m Logged By : CW/JC Reviewed By : JRW

Digger Type/Size : Bucket Excavator **Bucket Type/Size** : 400 mm

Latitude: -43.61536401997 **Longitude**: 172.40958556749

Excavatability (Relative Scale) DESCRIPTION DESCRIPTION							Buoket Type/Gize : 1								
Silty fine to medium SAND with trace gravel and rootlets; brown. Poorly graded [TOPSOIL] Sandy fine to coarse GRAVEL with minor cobbles; brownish grey. Well graded. Subrounded to subangular Sand, fine to coarse. Well graded. Cobbles, subrounded to subangular. GW 1.5 - Depth of Excavation: 2.3 m Termination Condition: met target depth M Depth of Excavation: 2.3 m Termination Condition: met target depth	Depth (m BGL)	Material	(Rel	cavatabi ative Sc	ale)	USCS Symbol	DESCRIPTION	Graphic Symbol	Elevation (mRL)	Water Level	Moisture Cond.	Consistency/ Density Index	Shear Vane Peak/Remolded (kPa)	Blows pe	er 100mm
cobbles; brownish grey. Well graded. Subrounded to subangular. Sand, fine to coarse. Well graded. Cobbles, subrounded to subangular. GW Tightty Packed Depth of Excavation: 2.3 m Termination Condition: met target depth							Silty fine to medium SAND with trace gravel and rootlets; brown. Poorly graded [TOPSOIL]	1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1			D	-			
Termination Condition: met target depth	1.0	ALLUVIUM				GW	cobbles; brownish grey. Well graded. Subrounded to subangular. Sand, fine to coarse. Well graded. Cobbles, subrounded to subangular.					Tightly Packed			
	2.5 -						Depth of Excavation: 2.3 m Termination Condition: met target depth				M				

undefined met target depth at 2.3 m depth . met practical refusal Standing groundwater was not encountered TS=TOPSOIL

TS=TOPSOIL

GEOTECH TEST PIT LOG TEST PIT LOGS.GPJ NZ MASTER DATA TEMPLATE.GDT 16/6/22



Geotechnical Investigation Broadfield Grange 572 Selwyn Road, Rolleston 18991.001.001 Client: Broadfield Grange LTD

Date: 02/06/2022

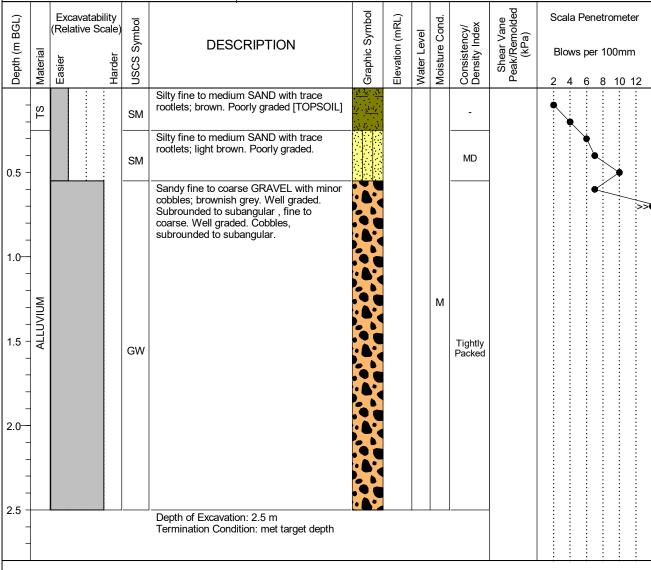
Shear Vane No : Logged By : CW/ JC

Max Test Pit Depth : 2.5 m

Logged By : CW/ JC Reviewed By : JRW

Digger Type/Size: Bucket Excavator **Bucket Type/Size**: 400 mm

Latitude: -43.614830032536 **Longitude**: 172.40905809273





Geotechnical Investigation Broadfield Grange 572 Selwyn Road, Rolleston 18991.001.001 Client : Broadfield Grange LTD

 Date
 : 02/06/2022
 I

 Max Test Pit Depth
 : 2.55 m
 Re

Digger Type/Size: Bucket Excavator **Bucket Type/Size**: 400 mm

Shear Vane No :

Logged By : JC/CW Reviewed By : JRW

Latitude: -43.613871542206 **Longitude**: 172.40920213943

Excavatability (Relative Scale) Page P							Bucket Type/Size . 4	100 111111					Longitude	. 1/2	4032	202 1	3343	
SM Silty fine to medium SAND with trace gravel and rootlets; brown. Poorly graded [TOPSOL]. SM Silty fine to medium SAND with trace gravel and rootlets; light brown. Poorly graded. Sandy fine to coarse GRAVEL with minor cobles; brownish gray. Well graded. Subrounded to subangular. Sand, fine to coarse. Well graded. Cobbles, subrounded to subangular. GW GW Tightly Packed Depth of Excavation: 2.55 m	Jepth (m BGL)	/aterial	(Relativ	/e Scale)	JSCS Symbol	DES	CRIPTION	Sraphic Symbol	:levation (mRL)	Vater Level	Aoisture Cond.	Sonsistency/ Density Index	Shear Vane Peak/Remolded (kPa)	Blo	ows pe	er 10	0mm	1
SM gravel and rootlets; light brown. Poorly graded. Sandy fine to coarse GRAVEL with minor cobbies; brownish grey. Well graded. Subrounded to subangular. 1.0 GW GW Depth of Excavation: 2.55 m	-					gravel and rootlets	m SAND with trace s; brown. Poorly graded	17 · 37 · 17 · 17	Ш	>		-		2	4 6	8	10 1	2
Sandy fine to coarse GRAVEL with minor cobbles; brownish grey. Well graded. Subrounded to subangular. 1.0- 1.5- Body fine to coarse GRAVEL with minor cobbles; brownish grey. Well graded. Subrounded to subangular. M Tightly Packed 2.0- Depth of Excavation: 2.55 m	-				SM	gravel and rootlets						MD			\	•		
Termination Condition: met target depth	1.0—	ALLUVIUM			GW	cobbles; brownish Subrounded to su coarse. Well grad subrounded to su	grey. Well graded. bangular. Sand, fine to ed. Cobbles, bangular.				М	Tightly Packed						>
						Depth of Excavati Termination Cond	on: 2.55 m lition: met target depth											

GEOTECH TEST PIT LOG TEST PIT LOGS.GPJ NZ MASTER DATA TEMPLATE.GDT 16/6/22



Geotechnical Investigation Broadfield Grange 572 Selwyn Road, Rolleston 18991.001.001 Client : Broadfield Grange LTD Shear Vane No :
Date : 02/06/2022 Logged By : CW

Max Test Pit Depth : 2.3 m Reviewed By : JRW

Digger Type/Size: Bucket ExcavatorLatitude: -43.61330885747Bucket Type/Size: 400 mmLongitude: 172.40898696182

				Bucket Type/Size . 4	00 111111					Longitude	172.40090090102	
Depth (m BGL)	Material	Excavatabilit (Relative Sca	Harder (a) < C	DESCRIPTION	Graphic Symbol	Elevation (mRL)	Water Level	Moisture Cond.	Consistency/ Density Index	Shear Vane Peak/Remolded (kPa)	Scala Penetromete Blows per 100mm 2 4 6 8 10 1	1
-	TS		SM	Silty fine to medium SAND with trace rootlets; brown. Poorly graded [TOPSOIL].				_	-			
0.5 -			SM	Silty fine to medium SAND with trace rootlets; light brown with orange mottles. Poorly graded. Rootlets no longer encountered					MD			
1.0-	ALLUVIUM			Sandy fine to coarse GRAVEL with minor cobbles and trace boulders; brownish grey. Well graded. Subrounded to subangular. Sand, fine to coarse. Well graded. Cobbles, subrounded to subangular. Less fines for 200mm.				М				>>
2.0	-		GW	Less fines for 200mm Trace silt encountered from 1.8 m depth					Tightly Packed			
2.5 -	-			Depth of Excavation: 2.3 m Termination Condition: met practical refusal								

undefined met practical refusal at 2.3 m depth due to hole collapse.

Standing groundwater was not encountered TS=TOPSOIL

TS=TOPSOIL

GEOTECH TEST PIT LOG TEST PIT LOGS.GPJ NZ MASTER DATA TEMPLATE.GDT 16/6/22



Geotechnical Investigation Broadfield Grange 572 Selwyn Road, Rolleston 18991.001.001

Client: Broadfield Grange LTD Client Ref. : N/A Date : 02/06/2022 Hole Depth : 0.3 m

Shear Vane No : $\textbf{Logged By}: \mathsf{JC/CW}$ Reviewed By : JRW

Latitude: -43.617111258606

		1	8991.001.001	Hole Diam	eter : 50	0 mm					ngitud	le : 172	2.410	84747	451
Depth (m BGL)	ərial	USCS Symbol	DESCRIPTION		Graphic Symbol	Elevation (mRL)	Water Level	Moisture Cond.	Consistency/ Density Index	Shear Vane Undrained Shear Strength (kPa) Peak/Remolded		Scala		trometo	
Dept	Material	OSC			Grap	Elev	Wate	Mois	Con	Str Str Pea	2	4			'' 12
	TOPSOIL	SM	Silty fine to medium SAND with transfer brown. Poorly graded [TOPSOIL].	ace rootlets;	\(\frac{\delta}{1} \)				-			•			
-	ALLUVIUM	SM	Silty fine to medium SAND; light b graded.	rown. Poorly				D	MD			•			
T			End of Hole Depth: 0.3 m Termination Condition: met praction	cal refusal	1								: '		. !
0.5 -															
1.0-															



Geotechnical Investigation Broadfield Grange 572 Selwyn Road, Rolleston 18991.001.001

Client: Broadfield Grange LTD Client Ref. : N/A Date : 02/06/2022 Hole Depth: 0.9 m

Shear Vane No : $\textbf{Logged By}: \mathsf{JC/CW}$ Reviewed By : JRW

> Latitude: -43.616021774451 Longitude: 172.41145793982

		1	8991.001.001	Hole De Hole Diame						Lor	atitude : -43.6160 ngitude : 172.411		
Depth (m BGL)	Material	USCS Symbol	DESCRIPTION		Graphic Symbol	Elevation (mRL)	Water Level	Moisture Cond.	Consistency/ Density Index	Shear Vane Undrained Shear Strength (kPa) Peak/Remolded	Scala Pene Blows per 2 4 6		 12
-	TOPSOIL	ML	Sandy SILT with trace gravel and in brown. Low plasticity. Sand, fine to Poorly graded [TOPSOIL].	rootlets; o medium.					-				
- 0.5	ALLUVIUM	SM	Silty fine to medium SAND; light b orange mottles. Poorly graded.	rown with				M	L - MD				
1.0—			Becomes wet and light brown from End of Hole Depth: 0.9 m Termination Condition: met practic								•	•	
me	et pra	ctical	met practical refusal at 0.9 m depth refusal undwater was not encountered	on inferred grav	/el.								



Geotechnical Investigation Broadfield Grange 572 Selwyn Road, Rolleston 18991.001.001

Client: Broadfield Grange LTD Client Ref. : N/A Date : 02/06/2022 Hole Depth : 0.3 m

 $\textbf{Logged By}: \mathsf{JC/CW}$ Reviewed By : JRW

Shear Vane No :

Latitude: -43.615261941016

		1	8991.001.001	Hole Diame						Lor	ngitude :		
Depth (m BGL)	ial	USCS Symbol	DESCRIPTION		Graphic Symbol	Elevation (mRL)	Water Level	Moisture Cond.	Consistency/ Density Index	Shear Vane Undrained Shear Strength (kPa) Peak/Remolded		netrome	
Depth	Material	nscs			Graph	Elevat	Water	Moist	Consis	She Undra Strer Peak	Blo 2 4	er 100m 8 10	m 0 12
	TOPSOIL	ML	Sandy SILT with minor rootlets; br plasticity. Sand, fine to medium. P [TOPSOIL].	oorly graded	11 . 3 12 . 3 14			M	-				
	ALLUVIUM	SM	Silty fine to medium SAND with tra light brown. Poorly graded.	ace gravel;					MD				
	<u> </u>		End of Hole Depth: 0.3 m Termination Condition: met practic	al refusal	<u> </u>	1							>
0.5 -													
-													
-													
1.0													
			net practical refusal at 0.3 m depth	on inferred fill.								 	



Geotechnical Investigation Broadfield Grange 572 Selwyn Road, Rolleston 18991.001.001 Client : Broadfield Grange LTD
Client Ref. : N/A
Date : 02/06/2022
Hole Depth : 0.3 m

Logged By: JC/CW Reviewed By: JRW Latitude: -43.6159

Shear Vane No :

Latitude: -43.61593406284 **Longitude**: 172.41049599847

		1	8991.001.001	Hole Diame						Lor	ngitud					7
Depth (m BGL)	Material	USCS Symbol	DESCRIPTION		Graphic Symbol	Elevation (mRL)	Water Level	Moisture Cond.	Consistency/ Density Index	Shear Vane Undrained Shear Strength (kPa) Peak/Remolded		Blow	Pene	100n	nm	
_ <u>a</u>	TOPSOIL	ML	Sandy SILT with trace gravel and brown. Low plasticity. Sand, fine to Poorly graded [TOPSOIL].	rootlets; o medium.	2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2		M	M		2%4	•	4	6	8 1	10	12
0.5			End of Hole Depth: 0.3 m Termination Condition: met praction	al refusal												>>
1.0-																

GEOTECH HAND AUGER HAND AUGER LOGS.GPJ NZ DATA TEMPLATE 2.GDT 16/6/22

Hand Auger met practical refusal at 0.3 m depth on inferred gravel. met practical refusal

Standing groundwater was not encountered



Geotechnical Investigation Broadfield Grange 572 Selwyn Road, Rolleston 18991 001 001

Client: Broadfield Grange LTD Client Ref. : N/A Date : 02/06/2022 Hole Depth : 0.25 m

Shear Vane No : $\textbf{Logged By}: \mathsf{CW/JC}$ Reviewed By : JRW

Latitude: -43.614766221441

	1	8991.001.001	Hole Diame						Lor			4098499	
Depth (m BGL) Material	USCS Symbol	DESCRIPTION	I	Graphic Symbol	Elevation (mRL)	Water Level	Moisture Cond.	Consistency/ Density Index	Shear Vane Undrained Shear Strength (kPa) Peak/Remolded			enetrome	
Depth (m Material	USC			Grap!	Eleva	Wate	Moist	Cons	Sh Undr Stre Peak	2	Blows p	per 100m 8 1	nm 0 12
TOPSOIL	SM	Silty fine to medium SAND with tr rootlets; brown. Poorly graded [TC	ace gravel and DPSOIL].				М	-		•	\		
4	SM	Silty fine to medium SAND with tr light brown. Poorly graded.	ace gravel;					MD	_				
0.5 -		End of Hole Depth: 0.25 m Termination Condition: met practi	cal refusal										
1.0-													



Geotechnical Investigation Broadfield Grange 572 Selwyn Road, Rolleston 18991.001.001

Client: Broadfield Grange LTD Client Ref. : N/A Date : 02/06/2022 Hole Depth : 0.3 m

Shear Vane No : $\textbf{Logged By} : \mathsf{CW/JC}$ Reviewed By : JRW

Latitude: -43.6143765184

		- 1	8991.001.001	Hole Diame	eter : 50	0 mm					ngitud	e : 17	72.409	93112	6242
n BGL)		ymbol	DESCRIPTION	ı	Symbol	n (mRL)	evel	Cond.	ancy/ Index	Shear Vane Undrained Shear Strength (kPa) Peak/Remolded		Scala	Pene	etrome	eter
Depth (m BGL)	Material	USCS Symbol			Graphic Symbol	Elevation (mRL)	Water Level	Moisture Cond.	Consistency/ Density Index	Shear Undrain Strengt Peak/Ro	2	Blow 4	s per		nm 0 1:
_	TOPSOIL	SM	Silty fine to medium SAND with transfer brown. Poorly graded [TOPSOIL].	ace rootlets;					-			•			
-	ALLUVIUM	SM	Silty fine to medium SAND with tra light brown. Poorly graded.	ace gravel;				D	MD				•		
		•	End of Hole Depth: 0.3 m Termination Condition: met praction	cal refusal	4.1,.										
-															
0.5 -															
											:	•			: :
-															
1.0—															



Geotechnical Investigation Broadfield Grange 572 Selwyn Road, Rolleston 18991.001.001

Client: Broadfield Grange LTD Client Ref. : N/A Date : 02/06/2022 Hole Depth : 0.25 m

 $\textbf{Logged By} : \mathsf{CW/JC}$ Reviewed By : JRW

Shear Vane No :

 $\pmb{\textbf{Latitude}: -43.613871735403}$

		1	8991.001.001	Hole Diame						Lor	ngitude: 1		
Depth (m BGL)	ial	USCS Symbol	DESCRIPTION		Graphic Symbol	Elevation (mRL)	Water Level	Moisture Cond.	Consistency/ Density Index	Shear Vane Undrained Shear Strength (kPa) Peak/Remolded		etromet	
Depth	Material	nscs			Graph	Elevat	Water	Moist	Consis	She Undra Strer Peak/	Blov 2 4	100mn 8 10	n 12
	TOPSOIL	SM	Silty fine to medium SAND with tra rootlets; brown. Poorly graded [TO	ace gravel and PSOIL].	77 97 97 97 97 97 97 97 97 97 97 97 97 9			М	-		•		
			End of Hole Depth: 0.25 m		11/2: 11/								· · · ·
_			End of Hole Depth: 0.25 m Termination Condition: met practic	al refusal									
0.5 -													
_													
_													
_	-												
1.0—	_												
_													
													:
me	et pra	ctical	net practical refusal at 0.25 m depth refusal undwater was not encountered	n on inferred gra	avel.								



APPENDIX 4:

ECan Boreholes



Bore or Well No	M36/7976
Well Name	SELWYN ROAD
Owner	WEATHERBY ESTATE LIMITED



	M00/7070	·	0000/0007
Well Number	M36/7976	File Number	CO6C/23897
Owner	WEATHERBY ESTATE LIMITED	Well Status	Active (exist, present)
Street/Road	SELWYN ROAD	NZTM Grid Reference	BX23:52386-70950
Locality	BROADFIELD	NZTM X and Y	1552386 - 5170950
Location Description		Location Accuracy	10 - 50m
CWMS Zone	Selwyn - Waihora	Use	Domestic and Stockwater,
Groundwater Allocation Zone	Selwyn-Waimakariri	Water Level Monitoring	
Depth	36.00m	Water Level Count	0
Diameter	150mm	Initial Water Level	10.60m below MP
Measuring Point Description	ToC	Highest Water Level	
Measuring Point Elevation	37.00m above MSL (Lyttelton 1937)	Lowest Water Level	
Elevation Accuracy	< 2.5 m	First reading	
Ground Level	0.40m below MP	Last reading	
Strata Layers	11	Calc Min 80%	12.04m below MP (Estimated)
Aquifer Name		Aquifer Tests	0
Aquifer Type		Yield Drawdown Tests	1
Drill Date	24 Aug 2005	Max Tested Yield	2 l/s
Driller	Daly Water Wells Ltd	Drawdown at Max Tested Yield	3 m
Drilling Method	Rotary Rig	Specific Capacity	0.63 l/s/m
Casing Material	Steel	Last Updated	08 Nov 2013
Pump Type		Last Field Check	
Water Use Data	No		

Screens

Screen No.	Screen Type	Top (m)	Bottom (m)	Slot Size (mm)	Slot Length (mm)	Diameter (mm)	Leader Length (mm)
1	Stainless steel	34	36				

Step Tests

Step Test Date	Step	Yield	Yield GPM	DrawDown	Step Duration
24 Aug 2005	1	2	26.396368	3.2	4

Comments

Comment Date	Comment
28 Mar 2006	Gridref changed from: M36:6233-3268

Bore Log

Borelog for well M36/7976

Grid Reference (NZTM): 1552387 mE, 5170951 mN

Location Accuracy: 10 - 50m

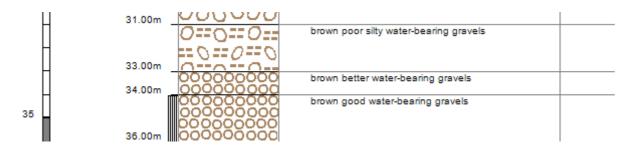
Ground Level Altitude: 36.6 m +MSD Accuracy: < 2.5 m

Driller: Daly Water Wells Ltd Drill Method: Rotary Rig

Borelog Depth: 36.0 m Drill Date: 24-Aug-2005



Scale(m)	Water Level	Depth(m)		Full Drillers Description	Formation Code
		0.30m -	66868	brown topsoil	
Н			000000000	grey dry clean gravels	
Н		0.50	000000000		
Ц		2.50m _ 3.00m _	70:00:00	grey sandy moist gravels	
			000000	brown claybound gravels	
5			000000		
			000000		
			000000		
H			000000		
H					
H			000000		
10			000000		
Н			000000		
Ц		12.00m _	200000		
Ц			0==0==0==	brown silty claybound gravels	
			==0==0		
15		15.00m	0==0==0==		
. [_	000000	grey heavy claybound gravels	
			000000		
Ħ			000000		
H			000000		
H					
20			000000		
Н		21.00m _	Dogue	grey sandy gravels traces of clay	
Н				grey samely graves makes or day	
Ц					
Ц			D:-0::0::		
25					
			P: 20 : 19:21		
		20.00	0::0::0		
		28.00m _	000000	brown claybound gravels	
			200000		
30					



Bore or Well No	M36/8002
Well Name	SELWYN ROAD
Owner	MR & MRS AT & JM MULCAY



Well Number	M36/8002	File Number	CO6C/24021
Owner	MR & MRS AT & JM MULCAY	Well Status	Active (exist, present)
Street/Road	SELWYN ROAD	NZTM Grid Reference	BX23:52536-70700
Locality	ROLLESTON	NZTM X and Y	1552536 - 5170700
Location Description		Location Accuracy	50 - 300m
CWMS Zone	Selwyn - Waihora	Use	Domestic and Stockwater,
Groundwater Allocation Zone	Selwyn-Waimakariri	Water Level Monitoring	
Depth	66.00m	Water Level Count	0
Diameter	150mm	Initial Water Level	12.40m below MP
Measuring Point Description	ToC	Highest Water Level	
Measuring Point Elevation	36.00m above MSL (Lyttelton 1937)	Lowest Water Level	
Elevation Accuracy	< 2.5 m	First reading	
Ground Level	0.30m below MP	Last reading	
Strata Layers	13	Calc Min 80%	11.96m below MP (Estimated)
Aquifer Name		Aquifer Tests	0
Aquifer Type		Yield Drawdown Tests	1
Drill Date	28 Oct 2005	Max Tested Yield	4 l/s
Driller	Smiths Welldrilling	Drawdown at Max Tested Yield	11 m
Drilling Method	Rotary/Percussion	Specific Capacity	0.37 l/s/m
Casing Material	Steel	Last Updated	08 Nov 2013
Pump Type		Last Field Check	
Water Use Data	No		

Screens

Screen No.	Screen Type	Top (m)	Bottom (m)	Slot Size (mm)	Slot Length (mm)	Diameter (mm)	Leader Length (mm)
1	Stainless steel	64.5	66				

Step Tests

Step Test Date	Step	Yield	Yield GPM	DrawDown	Step Duration
30 Oct 2005	1	4.06	53.5846252	11	2

Comments

Comment Date	Comment
04 May 2007	Gridref changed from: M36:6249-3232, original gridref incorrect (on road) still waiting for BCR

Bore Log

Borelog for well M36/8002

Grid Reference (NZTM): 1552537 mE, 5170701 mN

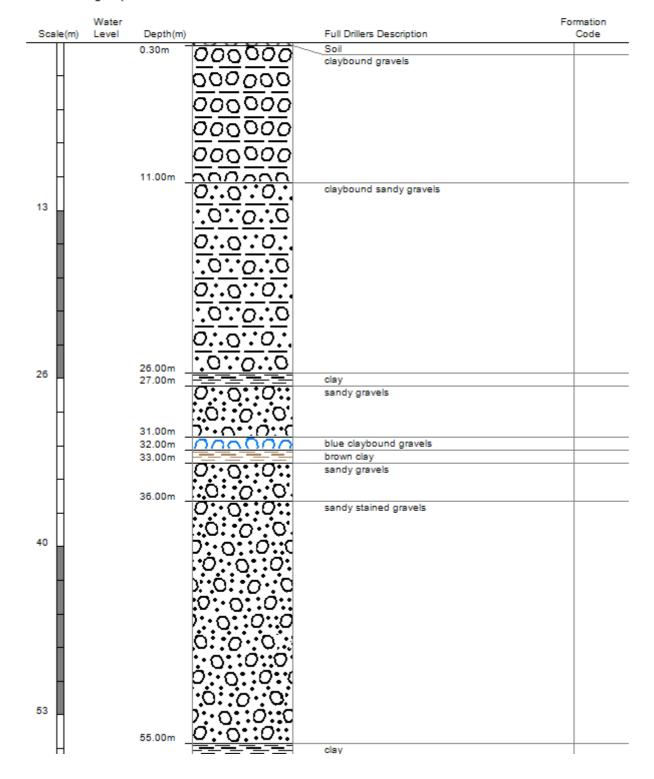
Location Accuracy: 50 - 300m

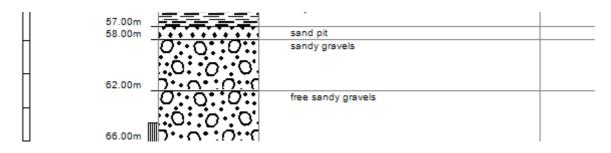
Ground Level Altitude: 35.7 m +MSD Accuracy: < 2.5 m

Driller: Smiths Welldrilling Drill Method: Rotary/Percussion

Borelog Depth: 66.0 m Drill Date: 28-Oct-2005







Bore or Well No	M36/8009
Well Name	SELWYN ROAD
Owner	MR & MRS GC & PM MORGAN



Well Number	M36/8009	File Number	CO6C/24231
Owner	MR & MRS GC & PM MORGAN	Well Status	Active (exist, present)
Street/Road	SELWYN ROAD	NZTM Grid Reference	BX23:52356-70860
Locality	BROADFIELD	NZTM X and Y	1552356 - 5170860
Location Description		Location Accuracy	50 - 300m
CWMS Zone	Selwyn - Waihora	Use	Domestic and Stockwater,
Groundwater Allocation Zone	Selwyn-Waimakariri	Water Level Monitoring	
Depth	36.00m	Water Level Count	0
Diameter	150mm	Initial Water Level	12.20m below MP
Measuring Point Description	ТоС	Highest Water Level	
Measuring Point Elevation	36.18m above MSL (Lyttelton 1937)	Lowest Water Level	
Elevation Accuracy	< 5 m	First reading	
Ground Level	0.40m below MP	Last reading	
Strata Layers	10	Calc Min 80%	11.92m below MP (Estimated)
Aquifer Name		Aquifer Tests	0
Aquifer Type		Yield Drawdown Tests	1
Drill Date	01 Nov 2005	Max Tested Yield	2 l/s
Driller	Daly Water Wells Ltd	Drawdown at Max Tested Yield	3 m
Drilling Method	Rotary Rig	Specific Capacity	0.63 l/s/m
Casing Material	Steel	Last Updated	08 Nov 2013
Pump Type		Last Field Check	
Water Use Data	No		

Screens

Screen No.	Screen Type	Top (m)	Bottom (m)	Slot Size (mm)	Slot Length (mm)	Diameter (mm)	Leader Length (mm)
1	Slotted PVC	34	36				

Step Tests

Step Test Date	Step	Yield	Yield GPM	DrawDown	Step Duration
01 Nov 2006	1	2	26.396368	3.2	3

No comments for this well

Bore Log

Borelog for well M36/8009 page 1 of 2

Grid Reference (NZTM): 1552357 mE, 5170861 mN

Location Accuracy: 50 - 300m

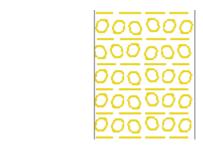
Ground Level Altitude: 35.8 m +MSD Accuracy: < 0.5 m

Driller: Daly Water Wells Ltd Drill Method: Rotary Rig

Borelog Depth: 36.0 m Drill Date: 01-Nov-2005



Scale(m)	Water Level	Depth(m)		Full Drillers Description	Formation Code
		0.25m		brown top soil	
- 11		0.25m		brown top soil	
- 11		0.60m	00000000	yellow/brown dry clay	
Н		0.60m	0000000000	yellow/brown dry clay grey dry gravels small-large (swl =	
- 11			000000000	11.8m)	
- 11			000000000		
Н			100000000000		
- 11			0000000000		
- 11			00000000		
Н			000000000		
- 11			000000000000000000000000000000000000000		
- 11			ĎÕÕÕÕÕÕÕÕÕ		
Н			0000000000		
- 11			000000000		
- 11		4.70m _	IOOOOOOO	grey dry gravels small-large (swl =	
5		4.70m	000000	11.8m)	
- 1			000000	yellow small-medium claybound gravels	
Н			000000		
- 1			000000		
Н			000000		
			000000		
H			000000		
			000000		
- H			70000		
- 1			000000		
10			200000		
			000000		
			000000		
П			000000		
			500,505		
Н			00000		
			000000		
Н			000000		
			000000		
			500,500		
П			50000		
			000000		
15			000000 000000 000000		



Bore or Well No	M36/8392
Well Name	572 SELWYN ROAD
Owner	A J LLOYD



Well Number	M36/8392	File Number	CO6C/26653
Owner	A J LLOYD	Well Status	Active (exist, present)
Street/Road	572 SELWYN ROAD	NZTM Grid Reference	BX23:52567-70574
Locality	SPRINGSTON	NZTM X and Y	1552567 - 5170574
Location Description		Location Accuracy	2 - 15m
CWMS Zone	Selwyn - Waihora	Use	Domestic Supply,
Groundwater Allocation Zone	Selwyn-Waimakariri	Water Level Monitoring	
Depth	36.00m	Water Level Count	0
Diameter	150mm	Initial Water Level	10.00m below MP
Measuring Point Description		Highest Water Level	
Measuring Point Elevation	34.26m above MSL (Lyttelton 1937)	Lowest Water Level	
Elevation Accuracy	< 5 m	First reading	
Ground Level	0.00m above MP	Last reading	
Strata Layers	5	Calc Min 80%	11.83m below MP (Estimated)
Aquifer Name		Aquifer Tests	0
Aquifer Type		Yield Drawdown Tests	1
Drill Date	12 Jun 2007	Max Tested Yield	2 l/s
Driller	Daly Water Wells Ltd	Drawdown at Max Tested Yield	11 m
Drilling Method	Rotary Rig	Specific Capacity	0.18 l/s/m
Casing Material	Steel	Last Updated	08 Nov 2013
Pump Type		Last Field Check	
Water Use Data	No		

Screens

Screen No.	Screen Type	Top (m)	Bottom (m)	Slot Size (mm)	Slot Length (mm)	Diameter (mm)	Leader Length (mm)
1	Stainless steel	34	36				

Step Tests

Step Test Date	Step	Yield	Yield GPM	DrawDown	Step Duration
12 Jun 2007	1	2	26.396368	11	0

Comments

Comment Date	Comment
20 Sep 2007	Gridref changed from: M36:6247-3217, BCR confirms
12 Nov 2007	Gridref changed from: M36:6255-3216. New gridref from plan in BCR
09 Jun 2009	Gridref changed from: M36:62572-32149 - Site visit 5 June 09, location GPS'd

Bore Log

Borelog for well M36/8392

Grid Reference (NZTM): 1552568 mE, 5170575 mN

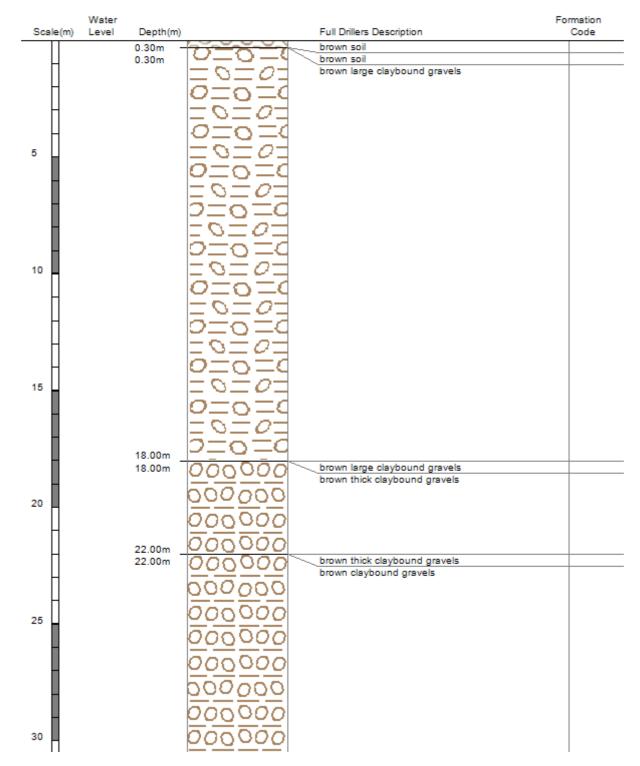
Location Accuracy: 2 - 15m

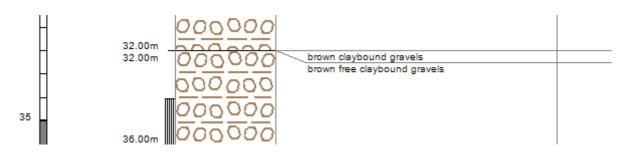
Ground Level Altitude: 34.3 m +MSD Accuracy: < 0.5 m

Driller: Daly Water Wells Ltd Drill Method: Rotary Rig

Borelog Depth: 36.0 m Drill Date: 12-Jun-2007









APPENDIX 5:

Statement of Professional Opinion



Statement of Professional Opinion on the Suitability of Land for Subdivision

(Appendix I to the Infrastructure Design Standard)

Issued by: ENGEO Ltd

(Geotechnical engineering firm or suitably qualified engineer)

To: Broadfield Grange Limited

(Owner/Developer)

To be supplied to: Selwyn District Council

(Territorial authority)

In respect of: Residential Subdivision (Description of proposed infrastructure/land development)

At: 1/572 Selwyn Road (Lot 2 DP 337894) and 4/572 Selwyn Road (Lot 4 DP 337894) (Address)

I (Geotechnical engineer) Don Bruggers on behalf of (Geotechnical engineering firm) ENGEO Ltd

hereby confirm:

- 1. I am a suitably qualified and experienced geotechnical engineer and was retained by the owner/developer as the geotechnical engineer on the above proposed development.
- 2. My/the geotechnical assessment report, dated 16 June 2022 has been carried out in accordance with the Department of Building and Housing Guidelines for geotechnical investigation and assessment of subdivisions and includes:
 - (i) Details of and the results of my/the site investigations.
 - (ii) A liquefaction assessment.
 - (iii) An assessment of rockfall and slippage, including hazards resulting from seismic activity.
 - (iv) An assessment of the slope stability and ground bearing capacity confirming the location and appropriateness of building sites.
 - (v) Recommendations proposing measures to avoid, remedy or mitigate any potential hazards on the land subject to the application, in accordance with the provisions of Section 106 of the Resource Management Act 1991.
- 3. In my professional opinion, I consider that Council is justified in granting consent incorporating the following conditions:

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As per our Geotechnical Investigation Report dated 16 June 2022 (reference: 18991.001.001 01).
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- 4. This professional opinion is furnished to the territorial authority and the owner/developer for their purposes alone, on the express condition that it will not be relied upon by any other person and does not remove the necessity for the normal inspection of foundation conditions at the time of erection of any building.
- 5. This certificate shall be read in conjunction with my/the geotechnical report referred to in Clause 2 above, and shall not be copied or reproduced except in conjunction with the full geotechnical completion report.
- 6. The geotechnical engineering firm issuing this statement holds a current policy of professional indemnity insurance of no less than \$ 1,000,000

(Minimum amount of insurance shall be commensurate with the current amounts recommended by IPENZ, ACENZ, TNZ, INGENIUM.)

(Signature of Engineer)

Date: 16/06/2022

Qualifications and experience: CMEngNZ (CPEng)